

STATE OF TENNESSEE DEPARTMENT OF TRANSPORTATION

TENNESSEE FLOWABLE FILL STUDY

FINAL REPORT

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16. Abstract

A study of Controlled Low-Strength Material (CLSM) was conducted in two parts: Excavatable flowable fill (EFF) and ZOOM! CLSM;

EFF

Twenty-three different EFF mixtures were placed in trenches simulating utility cuts during March through May of 2001. All EFF mixtures were tested for flow, unit weight, gravimetric air content, suitability for load application, and compressive strength development over time. The trenches were excavated in March 2003.

Nine EFF mixtures were used to access the impact of Portland cement content and ASTM C 618 Class F fly ash content. Portland cement contents of 30, 45, and 60 lbs/CY and ASTM C 618 Class F fly ash contents of 300, 370, and 440 lbs/CY were used to evaluate the impact of component proportions. PC-F Ash EFF mixtures with cementitious materials contents between 415 and 500 lbs/CY were found to have a high probability of having an ASTM D 6103 flow greater than 8-inches and passing the ASTM D 6024 ball drop test in less than 24 hours. In addition, the PC content of PC-Class F fly ash EFF mixtures appears to be more important to early compressive strength development. Further, strong relationships exist between component material proportions and 28-day and maximum compressive strengths and excavation difficulty.

Six EFF mixtures were used to access the impact of Portland cement content and high-unburned carbon fly ash content. Portland cement contents of 45 and 60 lbs/CY and high-unburned carbon fly ash contents of 370, 440, 510 lbs/CY were used to evaluate the impact of component proportions. The use of high carbon ash typically extended the time required to pass the ball drop test and often generated some erratic strength results. Therefore, the research team was hesitant to recommend the use of mixtures containing high carbon ash without further research.

The influence of aggregate type on EFF mixtures was evaluated by using five different aggregate types in the EFF mixture recommended by TRMCA (45 lbs/CY Portland cement and 370 lbs/CY ASTM C 618 Class F fly ash). Unfortunately, the previously mentioned strong relationships between component material proportions and 28-day and maximum compressive strengths and excavation difficulty appear to be aggregate dependent. In addition, very uniform fine aggregates were found to be likely to produce EFF mixtures that bleed excessively and are prone to segregation and flow problems. Further, limestone acreenings produced the highest 28-day and maximum compressive strengths and river sand produced the lowest 28-day and maximum compressive strengths in PC-F Ash EFF mixtures.

Four comparison EFF mixtures were also used in the study (1 TDOT and 3 air-entrained EFF mixtures). There appears to be an excellent relationship between compressive strength of air-entrained EFF mixtures and excavation difficulty. Further, air-entrained EFF mixtures are easier to excavate at the same compressive strength than non-air-entrained PC-Class F fly ash EFF mixtures. Finally, the Excavation Index (EI) combines the effects of compressive strength and density to predict excavatability. EFF mixtures with EI less than 10.0 are excavatable with a Case 580E backhoe.

ZOOM! CLSM

A high-flow, rapid-set, non-excavatable controlled low-strength material (flowable fill) was developed for applications where time was critical. The mixture was required to be ready for load application in six hours regardless of subgrade moisture conditions and uses materials commonly available at ready mix facilities. Further, the mixture, called ZOOM!, must be able to tolerate a wide variety of Tennessee fine aggregates types including Ohio River sand, manufactured limestone sand, limestone screenings, and crushed sandstone.

An initial ZOOM! mixture was produced using Ohio River sand. Subsequently, the mixture proportions were adjusted to produce the desired plastic and hardened properties with other Tenuessee fine aggregates. Laboratory work and three successful field demonstrations indicated that fine aggregate properties such as gradation and angularity dictate mixture proportions required to achieve flow, air content, and bleeding characteristics of ZOOM! CLSM. Average air temperature was found to be indirectly proportional to time of suitability for load application. ZOOM! CLSM air content was found to be directly proportional to time of suitability for load application.

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BACKGROUND

The Combustion By-Products Recycling Consortium (CBRC), Tennessee Ready Mixed Concrete Association, and Kentucky Ready Mixed Concrete Association began a study of excavatable flowable fill (EFF) in 2001. CBRC funding was reduced by the US Department of Energy due to increases in spending for homeland defense. Subsequently, CBRC decided not to fund the second year of the TTU EFF study. The Tennessee Department of Transportation Materials and Tests Division agreed to continue the project as primary sponsor beginning in August 2002.

The American Concrete Institute Committee 229 (1) defines controlled low strength material (CLSM) as a self-compacted, cementitious material used primarily as backfill having a compressive strength of 1200-psi or less. CLSM is commonly referred to as flowable fill. Flowable fill component proportions and resulting compressive strength are a function of the desired application. Flowable fills with strengths less than approximately 150-psi are termed excavatable and are primarily used as utility cut backfills. Flowable fills in the range of 150 to 1200-psi are used as structural fills, pavement base materials, or as a working platform.

The wide scope of TDOT operations encompasses applications for both EFF and non-excavatable flowable fill. Therefore, the study was conducted in two parts: First, the EFF study initiated by CBRC, TRMCA, and KRMCA continued with TDOT replacing CBRC as primary sponsor. Second, a high flow, rapid setting CLSM to serve as a working platform and/or structural fill was developed. Due to the dual approach used in the Tennessee Flowable Study, the final report is divided into two sections: the first on EFF and a second section on ZOOM! CLSM.

EXCAVATABLE FLOWABLE FILL

EFF Introduction

Excavatable flowable fill (EFF) is a blend of Portland cement, fine aggregate, water, and admixtures. EFF is delivered in a ready mix truck, but EFF is not concrete. EFF was developed to serve as an alternate backfill for roadway utility cuts. The three primary advantages of EFF are:

- Improved worker safety requires no compaction, therefore workers spend less time in the utility trench
- No in-service settlement utility cut patches do not sink producing roadway hazards
- 3) Can be removed with conventional excavating equipment no jack hammering

Unlike Portland cement concrete (PCC), higher compressive strength is not beneficial for EFF. PCC requires a minimum strength to perform properly in structures. EFF requires both a minimum and maximum strength to perform properly. Minimum strength recommendations are to assure that EFF has adequate bearing capacity and does not settle (deform) excessively under load. Maximum strength recommendations are to assure that EFF can be removed with conventional excavating equipment.

EFF Research Significance

EFF mixture design requires a new mindset. The "stronger is better" idea does not work with EFF. Several well-meaning designers have produced "EFF" mixtures, which are not excavatable using the "stronger is better" idea. The paste portion of the mixture

(Portland cement, water, air, and admixtures) is critical to EFF performance. Proper paste proportions allow EFF mixtures to be fluid, develop adequate early strength, and yet not become so strong that it cannot be excavated later. The primary purpose of the project is to increase specifying agency confidence in EFF by providing data on excavatability and the impact of component materials on EFF engineering properties.

EFF Research Objectives

The objectives of the proposed research are as follows:

- Determine the long-term excavatability of flowable fill mixtures containing various quantities of Portland cement and ASTM C 618
 Class F fly ash under field conditions and correlate the findings with compressive strength development in the laboratory.
- 2) Determine the long-term excavatability of flowable fill mixtures containing various quantities of Portland cement and high-unburned carbon fly ash under field conditions and correlate the findings with compressive strength development in the laboratory.
- 3) Determine if varying aggregate type significantly influences the results of objective 1.
- 4) Prepare a model specification for EFF in Kentucky and Tennessee based on the results of the project. Distribute the results of the project and the model specification on a CD-ROM to municipal officials and other users of flowable fill in Kentucky and Tennessee through the Kentucky and Tennessee Ready Mixed Concrete Associations.

EFF Literature Review

Table 1 shows some specifications / recommendations for compressive strength limitations for excavatability.

Table 1. Compressive Strength Limitations (in psi) for EFF Excavatability

Organization	Compressive Strength Limits (psi)				
ACI 229R-99 (1)	Manual < 50 Mechanical 100-200				
National Ready Mixed Concrete Assoc. (2)	30-200				
Illinois DOT (3)	150 @ 180 days				
Georgia DOT (4)	100 @ 28 days				
Florida DOT (5)	100 @ 28 days				
Bhat and Lovell (6)	80 - 150 by hand				
Indiana Ready Mixed Concrete Assoc. (7)	150				
Virginia Ready Mixed Concrete Assoc. (8)	200				

rapidly or extensively to perform adequately. The obvious solution to this problem is to increase the amount of paste components in the mixtures. However, increasing the Portland cement and fly ash contents also increase EFF compressive strength and thus reduce its excavatability. The effect of a Portland cement increase on EFF strength becomes rapidly obvious and most designers have recognized this fact. However, the effect of a fly ash increase, particularly a Class F fly ash increase, on EFF strength is not rapidly evident. Meade, Hunsucker, and Stone (9) reported that the two-year strength of EFF containing over 350 lbs/CY of Class F fly ash may be four to five times the 28-day strength. The slow strength development is due to a pozzolanic reaction between Portland cement hydration

products and the Class F fly ash. The long term effect of the pozzolanic reaction on compressive strength is apparently not well understood by some EFF designers.

The paste content of the mixture may also be increased using EFF air generators.

EFF generators produce high contents of stable air bubbles in the mixture. The high content of stable air bubbles:

- Limits ultimate strength
- Improves workability
- Reduces segregation
- Reduces shrinkage and virtually eliminates bleeding
- Reduces unit weight

In 1996, Rogers Group Inc. sponsored a project to determine if aggregate production byproducts containing high dust contents were a viable EFF aggregate. It was determined that high-fines limestone screenings containing up to 21 percent passing the Number 200 Sieve could be used as aggregates for air-entrained EFF mixtures (10). Experience garnered from several EFF projects suggests that compressive strength development and excavation difficulty are a function of five factors:

- 1) Air content
- 2) Portland cement content
- 3) Supplementary cementitious materials character and content
- 4) Aggregate type and gradation
- 5) Water content

EFF Performance Criteria

Table 2 shows consensus performance criteria selected for this study.

Table 2. EFF Performance Criteria

Criteria	Limit
Flow	≥ 8-inch
Ball Drop	≤24 hour
28-day Compressive Strength	≥ 30-psi
Ultimate Compressive strength	≤150-psi

EFF Mixtures

Table 3 shows nine EFF mixtures used to assess the impact of Class F Fly Ash and Portland cement content. Proportions for the EFF mixtures were chosen using Kentucky Transportation Cabinet (11) and Tennessee Ready Mixed Concrete Association (TRMCA) recommendations as well as a previous Tennessee Technological University (TTU) capping research mixture (12). Portland cement contents ranged from 30 to 60 lbs/CY. Class F fly ash contents ranged from 300 to 440 lbs/CY. The fine aggregate used was river sand.

Table 4 shows six EFF mixtures used to assess the impact of Portland cement content and high-unburned carbon fly ash content. Portland cement contents of 45 and 60 lbs/CY and high-unburned carbon fly ash contents of 370, 440, 510 lbs/CY were used to evaluate the impact of component proportions. The fine aggregate used was river sand.

The influence of aggregate type on EFF mixtures was evaluated by using five different aggregate types in the TRMCA EFF mixture as shown in Tables 5 and 6. Two additional aggregate variable mixtures were planned:

- Trench 20 oily foundry sand and
- Trench 21 clayey foundry sand

Oily foundry sand aggregate was not available at the time of trench placement. Clayey foundry sand contained such large metal fragments that local ready mix producers would not allow it in their mixers. Therefore, neither of the trenches was placed. Four additional comparison EFF mixtures (Table 7) were also used in the study.

Table 3. EFF Mixtures with Type I PC, Class F Fly Ash, and River Sand

Mixture	Date Placed	PC (lbs/CY)	Fly Ash (lbs/CY)	Water (lbs/CY)	Aggregate (lbs/CY)	Flow (in)	Ball Drop (hrs)
1 KTC	3/12/01	30	300	550	3000	0	22
2	3/12/01	30	370	501	2560	0	21
3	3/12/01	30	440	491	2508	9.5	21
4	3/12/01	45	300	510	2603	10.5	20
5 TRMCA	3/12/01	45	370	499	2552	11	19
6	5/14/01	45	440	490	2499	11	20
7	5/16/01	60	300	508	2595	0	19
8	5/16/01	60	370	500	2538 .	0	18
9 TTU CAP	5/16/01	60	440	498	2497	18	19

Table 4. EFF Mixtures with Type I PC, High Unburned Carbon Fly Ash, and River Sand

Mixture	Date Placed	PC (lbs/CY)	Fly Ash (lbs/CY)	Water (lbs/CY)	Aggregate (lbs/CY)	Flow (in)	Ball Drop (hrs)
10	5/15/01	45	370	515	2697	0	19
11	5/15/01	60	370	509	2600	8.5	18
12	5/15/01	45	440	500	2557	14	71
13	5/15/01	60	440	494	2560	15.5	66
14	5/15/01	45	510	481	2527	15	69
15	5/15/01	60	510	479	2520	16	66

Table 5. Aggregate Variables for TRMCA (45 PC / 370 F Ash) EFF Mixture

Mixture Number / Fine Aggregate	Date Placed	Water (lbs/CY)	Aggregate (lbs/CY)	Flow (in)	Ball Drop (hrs)
5 River Sand	3/12/01	499	2552	11	19
16 Limestone Manufactured Sand	5/14/01	499	2552	11	48
17 Crushed Sandstone	5/14/01	593	2362	0	22
18 Masonry Sand	5/14/01	641	2190	12.5	44
19 Limestone Screenings	5/14/01	448	2611	10.5	23
20 Oily Foundry Sand*					<u> </u>
21 Clayey Foundry Sand*		<u> </u>			

^{* -} Not placed

Table 6. Variable Aggregate Specifications

Aggregate Type	Specification
Crushed sandstone	TDOT PCC Fine Agg. – near ASTM C 33 (13)
Manufactured Limestone Sand	TDOT PCC Fine Agg. – near ASTM C 33 (13)
Masonry Sand (high silica dredged sand)	Near ASTM C 144
Limestone Screenings	AASHTO M43 Size Number 10 (14)
Oily Foundry Sand*	None
Clayey Foundry Sand*	None

^{* -} Not placed

Table 7. EFF Comparison Mixtures

Mixture	Date Placed	PC (lbs/CY)	Fly Ash (lbs/CY)	Water (lbs/CY)	Agg. (lbs/CY)	Flow (in)	Air (%)	Ball Drop (hrs)
22 TDOT (15)	5/14/01	100	250	500	2800*	0		21
23 MB AE 90	5/15/01	100	0	340	2439*	8.75	28.3	46
24 WRG Darafill	5/15/01	100	0	270	2316*	6.25	24.3	20
25 MBT Rheofill	5/15/01	80	0	375.3	2501**	7.25	25.9	67

^{* -} River Sand, ** - Limestone Manufactured Sand

EFF Procedure

Approximately 5.5 cubic yards of each EFF mixture was delivered to the TTU campus in a ready mix truck. 5.33 cubic yards of each mixture was placed in a 3-feet deep, 3-feet wide, 16-feet long trench simulating a utility cut. The remainder of the mixture was used to cast compressive strength cylinders and conduct plastic property tests.

Each mixture was sampled near the middle of the batch in accordance with ASTM D 5791 (16). The consistency of each mixture was determined as per ASTM D 6103 (17). The unit weight and air content of each mixture were determined in accordance with ASTM D 6023 (18). Flow values and air contents (for air-entrained mixtures only) are shown in Tables 3, 4, 5, and 7. Fifty 4-inch diameter, 8-inch height, compressive strength cylinders of each mixture were cast in accordance with ASTM D 4832-95 (19) with the following exceptions:

- Cardboard molds were used rather than plastic due to stripping difficulties with CLSM in plastic molds;
- CLSM was not mounded on top of the cylinder in the plastic state and removed after hardening with a wire brush due to the high potential for cylinder damage.

All EFF compressive strength cylinders were initially cured in the molds with the tops covered with plastic bags secured by rubber bands. The cylinders were stored in wooden cabinets that were exposed to typical laboratory conditions (temperature and humidity). Unpublished preliminary research with some of the same EFF mixture designs had shown that the cylinders disintegrated when submerged in water. When testing the cylinders at 546 days of age, many of the specimens had become extremely

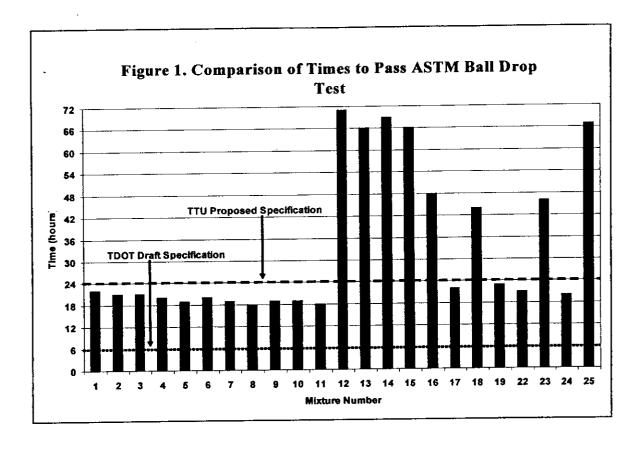
dry and brittle. It was determined that sufficient samples remained to submerge one-half of them in a lime-water curing tank for the duration of the project to determine the effect submerged curing at a late age would have on compressive strength. Compressive strength tests at 637 and 728 days used both samples that were cured in the molds and samples that were immersed at 546 days age. Complete compressive strength data can be found in Appendix A. In the body of the report on the larger of the two average compressive strengths was reported and used for correlations.

Three compressive strength cylinders were tested at each time shown in Table 8, providing that sufficient cylinders survived transportation and mold stripping. The compressive strength testing was conducted in accordance with ASTM D 4832-95 with the following exception. Compressive strength cylinders were capped with wet-suit neoprene in rigid retaining caps as described by Sauter and Crouch (12).

Table 8. Testing Times

Days	7	28	63	98	140	182	238	301	364	455	546	637	728
Weeks	1	4	9	14	20	26	34	43	52	65	78	91	104
Years						0.5			11	1.25	1.5	1.75	2

The EFF trenches were tested for suitability for load application at approximately 6 hours after placement and subsequently every 2 to 4 hours during regular work hours until each mixture passed the test or 7 days elapsed. The test was conducted as prescribed in ASTM D 6024 (20). The time each trench passed ASTM 6024 is shown in Tables 3, 4, 5, and 7 and Figure 1.



Two Dynamic Cone Penetrometer (DCP) tests (21) were conducted on each trench on Monday 3/10/03. Several attempts were made to push Shelby Tubes and obtain compressive strength samples; however, no viable samples were recovered. Two attempts to excavate the EFF in each trench with a Case 580E backhoe were made on Tuesday 3/11/03. The backhoe operator provided a 1 to 10 (10 hardest) estimate of excavation difficulty for each trench. In addition, two attempts to excavate each trench were made with a sharpshooter shovel. Further, two pick penetration tests were conducted on each trench. Trenches containing mixtures 1 through 5 were two years old at the time of testing and excavation, the remainder of the trenches were approximately 22 months old.

EFF Results

The average values of 28-day, maximum obtained, and compressive strengths at excavation (637 or 728-day) are shown in Table 9. Complete compressive strength results are available in Appendix A. Results of EFF trench testing just prior to excavation and excavation difficulties are shown in Table 10. The operator deemed all EFF mixtures excavatable. However, excavation difficulty varied considerably.

Table 9. 28-day, Maximum, and Excavation Compressive Strengths

Mixture	Aggregate	28-day Compressive Strength (psi)	Maximum Compressive Strength (psi)	Excavation (637 or 728) Compressive Strength (psi)
1 KTC 30/300 F	River Sand	6	10	7
Mixture 2 30/370 F	River Sand	10	15	12
Mixture 3 30/440 F	River Sand	13	22	17
Mixture 4 45/300 F	River Sand	20	48	48
5 TRMCA 45/370 F	River Sand	17	41	29
Mixture 6 45/440 F	River Sand	20	68	68
Mixture 7 60/300 F	River Sand	17	99	48
Mixture 8 60/370 F	River Sand	27	90	66
9 TTU Cap 60/440 F	River Sand	40	145	128
10 45/370 HC	River Sand	34	94	84
11 60/370 HC	River Sand	41	144	101
12 45/440 HC	River Sand	20	30	26
13 60/440 HC	River Sand	31	83	64
14 45/510 HC	River Sand	29	68	58
15 60/510 HC	River Sand	29	137	132
16 45/370 F	LS Man. Sand	49	106	76
17 45/370 F	Crushed Sandstone	24	126	94
18 45/370 F	Masonry Sand	48	128	104
19 45/370 F	LS Screenings	58	134	119
22 TDOT 100/250	River Sand	40	201	159
23 MBT MB AE 90	River Sand	16	39	29
24 W. R. Grace Darafill	River Sand	36	78	55
25 MBT Rheofill	LS Man. Sand	17	46	41

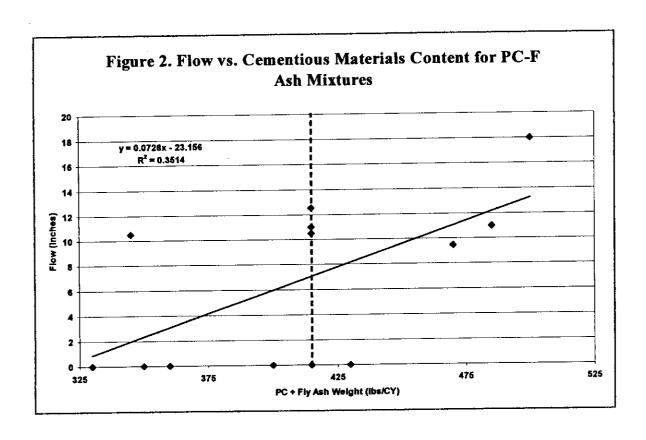
Table 10. Trench Testing and Excavation Results

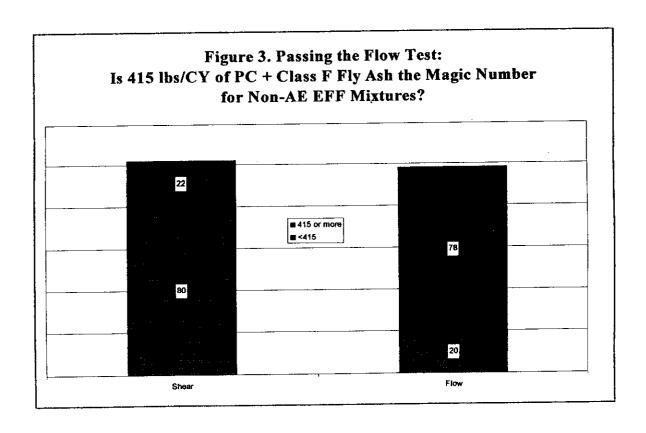
T			Case 580E Backhoe		Manual Excavation		
Mixture	Average Comp. Strength @ Excavation (psi)	Average Dynamic Cone Penetrometer (psf)	Average Hydraulic Line Pressure (psi)	Average Operator Rating (1-10)	Average Shovel Difficulty (1-10)	Average Pick Penetration (inches)	
1 KTC 30/300	7	5000	1450	1	1	1.85	
Mixture 2 30/370	12	>10,000	1668	3	10	1.65	
Mixture 3 30/440	17	>10,000	1740	4	10	2.1	
Mixture 4 45/300	48	>10,000	1813	7	9	1.35	
5 TRMCA 45/370	29	7500	1269	7	10	1.65	
Mixture 6 45/440	68	>10,000	1849	8	9	1.65	
Mixture 7 60/300	48	>10,000	1958	7	10	1.45	
Mixture 8 60/370	66	9000	1523	8	9	1.95	
9 TTU Cap 60/440	128	>10,000	1450	9	9	2.5	
10 45/370 HC	84	7500	1595	7	8	3.35	
11 60/370 HC	101	>10,000	1523	9	9	1.65	
12 45/440 HC	26	3250	1595	4	1	5.1	
13 60/440 HC	64	3500	2175	3	3	3.95	
14 45/510 HC	58	3250	2175	3	2	4.65	
15 60/510 HC	132	>10,000	2030	6	9	2.95	
16 LS Man. Sand 45/370	76	4500	2030	9	9	3.95	
17 Crushed SS 45/370	94	>10,000	1740	8	9	1.9	
18 Masonry Sand 45/370	104	8000	2248	7	7	2.55	
19 LS Scr. 45/370	119	6500	1813	9	10	2.7	
22 TDOT 100/250	159	>10,000	1595	10	10	0.65	
23 MBT MB AE 90	29	2750	2103	3	1	5.15	
24 W. R. Grace Darafili	55	5750	2030	6	9	3.1	
25 MBT Rheofill LS Man. Sand	41	2000	1885	4	1	6.5	

Analysis of EFF Results

Mixtures 9 (TTU Cap 60/440 river sand), 11 (60 / 370 high carbon ash, river sand), and 19 (TRMCA 45/370 limestone screenings) were the only three mixtures complying with all the criteria shown in Table 2. However, the use of high carbon ash typically extended the time required to pass the ball drop test. Mixtures 6 and 12 only differed in ash type, but mixture 12 took over three times as long to pass ball drop. Similarly, mixtures 9 and 13 only differed in ash type, but mixture 13 took over three times as long to pass ball drop. High carbon ash mixtures also generated some erratic strength results. Mixtures 12 and 13 should have been stronger than mixture 11 due to higher cementitious contents; however both were weaker than mixture 11. Therefore, the research team was hesitant to recommend the use of mixtures containing high carbon ash without further research.

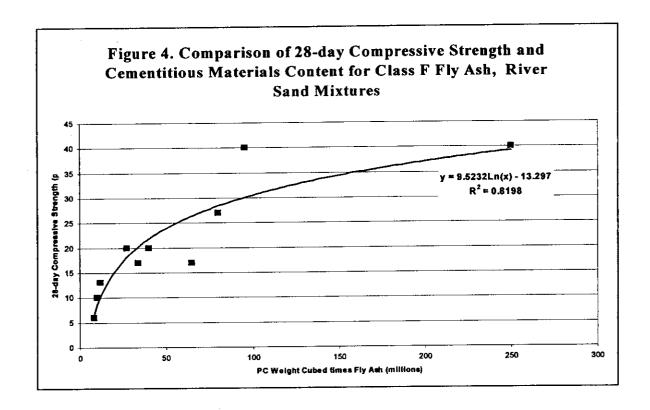
Figure 2 shows a correlation between ASTM D 6103 flow and cementitious materials content of the PC-ash mixtures. Although the coefficient of determination is low (0.3514), it is interesting to note that for EFF mixtures with a cementitious materials content of 415 lbs/CY or more, 78 percent had a flow greater than 8 inches. However, for EFF mixtures with cementitious materials content less than 415 lbs/CY, only 20 percent had a flow greater than 8 inches. Figure 3 shows a graphical illustration of importance of cementitious materials content to EFF flow.

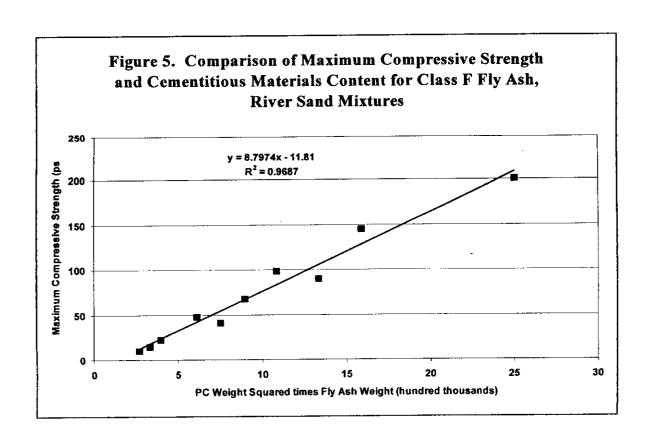




Time to pass the ASTM D 6024 Ball Drop Test for the PC-ash mixtures did not correlate well with mixture composition. However, 87.5 percent of non-air-entrained PC-F Ash EFF mixtures with cementitious materials contents less than or equal to 500 lbs/CY passed the ASTM D 6024 ball drop test in less than 24 hours. It was likely that high (> 500 lbs/CY) cementitious materials content inhibited bleeding of the EFF mixtures, thus retarding dewatering and subsequent stiffening. No correlation to time to pass the ball drop test was attempted with the limited number of air-entrained EFF mixtures.

Figure 4 shows a correlation between 28-day compressive strength and PC weight cubed times fly ash weight for Class F fly ash in River sand mixtures. The R² of 0.8198 indicates a fairly strong relationship. Figure 5 shows maximum compressive strength versus PC weight squared times fly ash content for Class F fly ash in River sand mixtures. The excellent fit (R² = 0.9687) of the linear trend line shows mathematically what the industry personnel have intuitively known for some time – EFF potential compressive strength is directly proportional to cementitious materials content. PC content of the PC-Class F fly ash EFF mixtures is more influential for early compressive strength development (28-day) than for maximum compressive strength as indicated by the cubed and squared PC weight relationships. Similar correlations for PC-High Unburned Carbon Ash were very poor, indicating no relationships.

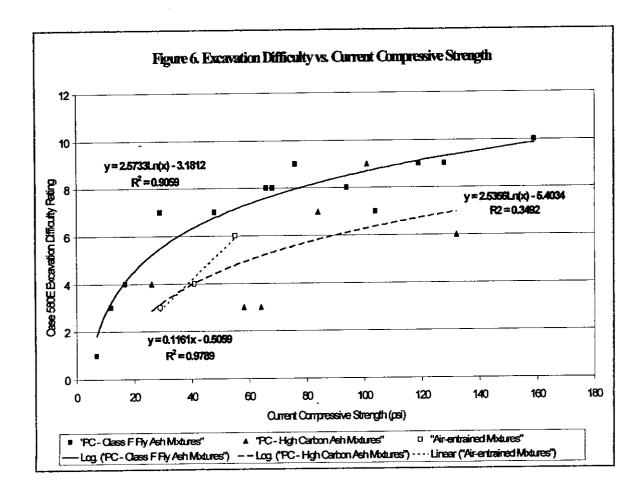




Substitution of variable aggregates for river sand into the TRMCA EFF mixture yielded few useful observations. Flow of all the mixtures was similar except for the crushed sandstone mixture. The locally available crushed sandstone often has more than 55 percent of the particles by weight between the No. 30 and No. 50 sieves, leading to a very open gradation prone to bleeding and segregation. No viable explanation could be developed for the difference in time to pass the ball drop. River sand reduced both 28-day and maximum compressive strengths. Limestone screenings produced the highest 28-day and maximum compressive strengths. Although Case 580E operator ratings for excavation difficulty for all the aggregate variable mixtures were similar, compressive strengths and other trench testing results varied widely. The importance of cementitious materials content to flow, time to pass the ball drop, and compressive strength development shown in Figures 2 through 5 suggest that the cementitious materials content of a PC-Class F fly ash EFF mixture is extremely important to mixture performance. Unfortunately, it appears that compressive strength cementitious materials relationships are aggregate dependent.

Figure 6 shows correlations between current compressive strengths at the time of excavation and Case 580E backhoe excavation difficulty. Good relationships were obtained for PC-Class F fly ash mixtures with all aggregate types included and for airentrained mixtures. Only three air-entrained mixtures were available for correlation. Airentrained mixtures are easier to excavate than non-air-entrained mixtures at the same compressive strength. A poor correlation was obtained with PC-high unburned carbon ash mixtures indicating no relationship. DCP, Case 580E hydraulic line pressure,

sharpshooter shovel, and pick penetration data did not correlate well will with mixture composition or excavation difficulty rating.



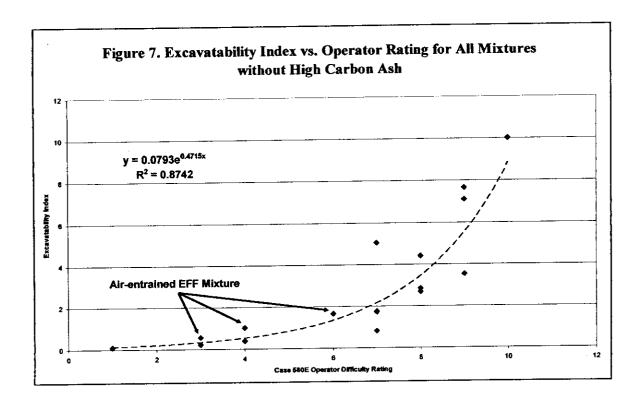
Mr. William Brewer, P.E., former chair of ACI Committee 229 on CLSM, developed a "Removability Modulus" for Hamilton County and the City of Cincinnati, Ohio. Brewer (22) showed that the excavatability of an EFF mixture is a function of compressive strength and unit weight. The findings of this project support that conclusion. The Excavatability Index (EI) is a modification of an equation developed by Mr. Brewer to fit the data obtained in this study. The modified equation shown below was developed such

that values less than 10.0 indicate that the EFF mixture is excavatable with a Case 580E backhoe.

EI = [(Comp. strength @ excavation in psi)^{1.5} x (plastic unit weight in pcf)^{1.285})]/100000

Table 9. Excavation Indices for EFF without High Carbon Ash

Mixture	Average Compressive Strength @ Excavation (psi)	Plastic Unit Weight (pcf)	Excavation Index
1 KTC 30/300	7	131.8	0.10
Mixture 2 30/370	12	135	0.23
Mixture 3 30/440	17	132.3	0.37
Mixture 4 45/300	48	131.6	1.76
5 TRMCA 45/370	29	131	0.82
Mixture 6 45/440	68	121.8	2.68
Mixture 7 60/300	48	130	1.73
Mixture 8 60/370	66	131.8	2.84
9 TTU Cap 60/440	128	131.6	7.66
16 LS Manufactured Sand 45/370	76	132.4	3.53
17 Crushed Sandstone 45/370	94	122.4	4.39
18 Masonry Sand 45/370	104	121	5.03
19 LS Screenings 45/370	119	135.2	7.11
22 TDOT 100/250	159	126	10.02
23 MBT MB AE 90	29	98.6	0.57
24 W. R. Grace Darafill	55	106.7	1.65
25 MBT Rheofill LS Manufactured Sand	41	103.8	1.02



EFF Conclusions

The following conclusions can be drawn from the limited data available:

- 1) Non-air-entrained PC-F Ash EFF mixtures with cementitious materials contents of 415 lbs/CY or greater have a much higher probability (78%) of achieving an ASTM D 6103 flow greater than 8 inches than similar EFF mixtures with lower cementitious materials contents (probability = 22%).
- 2) 85.7 percent of non-air-entrained PC-F Ash EFF mixtures with cementitious materials contents less than or equal to 500 lbs/CY passed the ASTM D 6024 ball drop test in less than 24 hours.
- 3) There appears to be a good relationship between 28-day compressive strength and PC weight cubed times fly ash weight for non-air-

- entrained PC-Class F fly ash EFF mixtures. Unfortunately, the relationship appears to be aggregate dependent.
- 4) There appears to be an excellent relationship between maximum (potential) compressive strength and PC weight squared times fly ash weight for non-air-entrained PC-Class F fly ash EFF mixtures.

 Unfortunately, the relationship appears to be aggregate dependent.
- 5) Uniform crushed sandstone fine aggregates with 55 percent or more of the particles by weight between the No. 30 and No. 50 sieves are likely to produce EFF mixtures that bleed excessively and are prone to segregation and flow problems.
- 6) Limestone screenings produced the highest 28-day and maximum compressive strengths in PC-F Ash EFF mixtures. River sand produced the lowest 28-day and maximum compressive strengths in PC-F Ash EFF mixtures.
- 7) The PC content of non-air-entrained PC-Class F fly ash EFF mixtures appears to be more important to early compressive strength development.
- 8) There appears to be a strong relationship between compressive strength of non-air-entrained PC-Class F fly ash EFF mixtures and excavation difficulty.

- 9) The use of high carbon ash typically extended the time required to pass the ball drop test and generated some erratic strength results. Therefore, the research team was hesitant to recommend the use of mixtures containing high carbon ash without further research.
- 10) There appears to be an excellent relationship between compressive strength of air-entrained EFF mixtures and excavation difficulty. Further, air-entrained EFF mixtures are easier to excavate at the same compressive strength than non-air-entrained PC-Class F fly ash EFF mixtures.
- 11) The Excavation Index (EI) combines the effects of compressive strength and density to predict excavatability. EFF mixtures with EI < 10.0 are excavatable with a Case 580E backhoe.

EFF Recommendations

1) The two mixtures shown in Table 10 are field-proven to comply with the criteria shown in Table 11.

Table 10. Field Proven EFF Mixtures

Mixture	Portland Cement	Class F Fly Ash	Aggregate	Water
TTU Cap River Sand	60	440	2492	498
Limestone Screenings	45	370	2611	448

Criteria	Limit	TTU Cap River Sand	Limestone Screenings
Flow	≥ 8-inch	18	10.5
Ball Drop	< 24 hours	19	23
28-day Compressive Strength	≥ 30-psi	40	58
Excavatability with Case 580E backhoe	≤159-psi	128	119
Excavatability Index	< 10.0	7.66	7.11

Table 11. Final Criteria and Results of Field Proven EFF Mixtures

2) For non-air-entrained PC-F Ash EFF mixtures use 415-500 lbs/CY of PC plus Class F fly ash.

 ≤ 10.0

EFF Technology Transfer

Excavatability Index

Technology transfer was accomplished by preparing a minimum of 1000 copies of a CD-ROM containing the project results and a model specification for EFF. The CD-ROMs were distributed by holding four seminars (Nashville 7/16/03 and Knoxville 7/17/03 in Tennessee, and Frankfort 8/13/03 and Owensboro 8/14/03 in Kentucky) for ready mix producers, municipal and state specifying officials. The CD-ROMs were also to be distributed by TRMCA and KRMCA. The CD-ROMs and associated seminars were funded by KRMCA. Additional technology transfer was given through a presentation at the ASTM Symposium on Innovations in Controlled Low-Strength Material (Flowable Fill) on June 19, 2003 in Denver, Colorado. A paper entitled "Long Term Study of 23 Excavatable Tennessee CLSM Mixtures" was submitted for possible publication in ASTM STP 1459 (at the discretion of the ASTM Committee on Publications).

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EFF DISCLAIMERS

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The opinions, findings, and conclusions expressed here are those of the authors and not necessarily those of the Tennessee Department of Transportation, the Tennessee Ready Mixed Concrete Association, or the Kentucky Ready Mixed Concrete Association.

ZOOM! CLSM

ZOOM! Introduction

The Tennessee Department of Transportation Division of Materials & Tests saw a need for a rapid set, non-excavatable Controlled Low-strength Material (CLSM) for applications where time was a critical factor such as rapid subgrade repairs. CLSM initial set can occur in two ways: dewatering and chemical reactions. The research team decided that the CLSM should initially harden due to chemical reactions rather than by bleeding (dewatering) in case of unfavorable placement conditions. The new CLSM needed to set and gain compressive strength rapidly yet have a very fluid consistency while plastic. The new CLSM mixture was named ZOOM! to reflect the rapid set and strength gain.

ZOOM! Research Objectives

Tennessee Technological University (TTU) researchers established the criteria shown in Table 12 for the ZOOM! mixture.

ZOOM! Materials

Type I Portland Cement meeting ASTM C 150 (23) was selected. Local tap water was also used for all laboratory mixtures. An air-entraining admixture, conforming to ASTM C 260-97 (24), was used in all laboratory ZOOM batches. A commercially available powder-form CLSM air generator was used for ZOOM! field batches. The commercially available high-range water reducer and the water-reducing accelerator conformed to ASTM C 494 (25) Types F and E, respectively.

Commercially available, single-use cardboard molds (4 by 8-inch), reported by the manufacturer to be in compliance with ASTM C 470 (26), were used for all compressive strength samples. Commercially available wet-suit neoprene pads in ASTM C 1231 (27) rigid retainers were used for capping all compressive strength specimens.

ZOOM! Proportions with Ohio River Sand (Control Fine Aggregate)

The initial ZOOM! mixture was proportioned by trial batches in the laboratory at TTU with Ohio River Sand fine aggregate. The Ohio River Sand used is the common fine aggregate for Portland cement concrete in West and Middle Tennessee. The initial ZOOM! mixture proportions are shown in Table 13 and the initial plastic properties are shown in Table 14. Compressive strength cylinders of each mixture were cast in accordance with ASTM D 4832-95 (19) with the following exceptions:

- Cardboard molds were used rather than plastic due to stripping difficulties with CLSM in plastic molds;
- CLSM were not mounded on top of the cylinder in the plastic state and removed after hardening with a wire brush due to the high potential for cylinder damage.

Three compressive strength cylinders were tested at each time providing that sufficient cylinders survived transportation and mold stripping. The compressive strength testing was conducted in accordance with ASTM D 4832-95 with the following exception. Compressive strength cylinders were capped with wet-suit neoprene in rigid retaining caps as described by Sauter and Crouch (12).

ASTM D 6024 testing required a large quantity of CLSM. Since the large quantity required would have been difficult to produce in the laboratory, the research team decided to correlate ball drop results from EFF mixtures 1, 3, 5, 7, 9 with their average compressive strengths at the time each mixture passed the ball drop test. One-half cubic-foot batches were mixed and three cylinders made for each of the five EFF mixture designs. The cylinders were tested for compressive strength at the time each EFF mixture passed the ball drop test. It was thus determined that to pass the test for load application, ZOOM! CLSM needed to have a compressive strength greater than 6-psi at six hours.

Other Fine Aggregates for ZOOM!

Three additional fine aggregates were selected to represent the fine aggregate types commonly available across the state of Tennessee. Manufactured Limestone Sand is commonly used in East Tennessee. In addition, some concrete producers in the Cumberland Plateau Region use crushed sandstone fine aggregate. Limestone screenings were included due to their abundance in Middle and East Tennessee. The use of screenings would help address an industry-wide problem as well as reduce ZOOM! cost if the screenings proved to be a viable aggregate. Fine aggregate gradations, determined as per AASHTO T 11-96 (28) and AASHTO T 27-99 (29) are shown in Figure 8. Uncompacted void values, determined in accordance with AASHTO T 304-96 (30), are shown in Figure 9. Fine aggregate specific gravities and absorptions, determined in accordance with AASHTO T84-00 (31), are shown in Table 15.

Table 12. ZOOM! CLSM Mixture Criteria

Parameter	Requirement	
Application	Rapid-set structural fill or working platform	
ASTM D 6024 Ball Drop Test (20)	Pass in 6 hours or less regardless of subgrade moisture	
Bleeding	Little or no bleeding	
Shrinkage	Little or no shrinkage	
ASTM D 6103 Flow (17)	8.75-inches minimum	
ASTM D 6023 Air Content (18)	Prefer 20 to 30 percent	
ASTM D 4832 compressive strength (19)*	30-psi minimum at 24-hours	
Excavatability	No requirement	
Aggregates	Producible with a wide variety of Tennessee aggregates	

^{* -}revised as recommended by Sauter and Crouch (12)

Table 13. Initial ZOOM! CLSM Plastic Properties with Ohio River Sand Control Aggregate

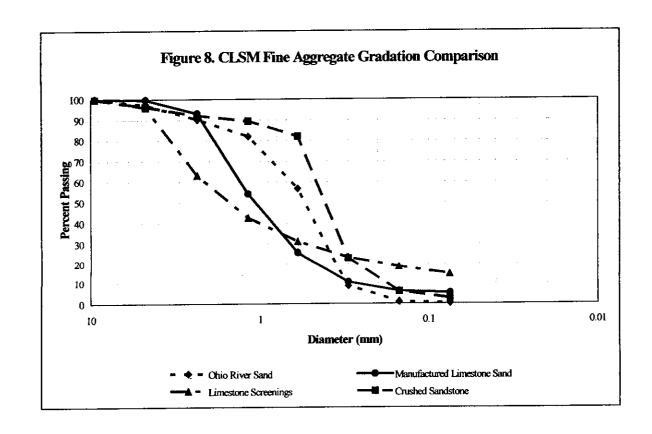
Property	Ohio River Sand (Control Aggregate)	Requirement	
Flow (inches)	8.5	8.75 minimum	
Bleed Time (min)	4.5	Little or no bleeding	
Shrinkage	Minimal	Little or no shrinkage	
Air Content (%)	25.7	20 to 30 preferred	
Unit Weight (pcf)	104.6	No requirement	
Meet Requirements / Problems	No Best combination of flow and bleeding achievable		

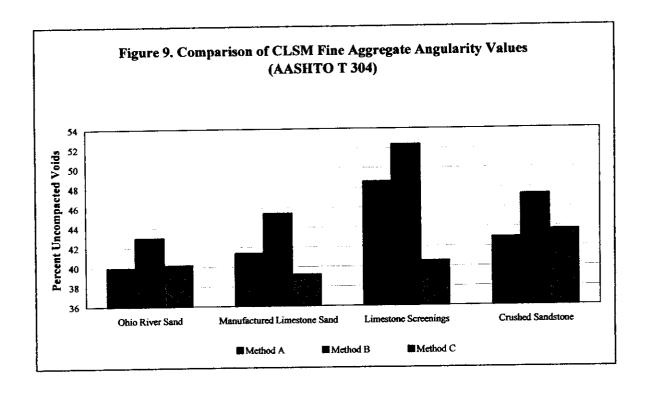
Table 14. Initial ZOOM! CLSM Mixture Proportions Developed from Trial Batches with Ohio River Sand Control Aggregate

Component	Amount
Type 1 Portland cement	300 lbs/CY
Water	317 lbs/CY
Ohio River Sand (SSD)	2425 lbs/CY
Air-entraining agent	70 oz/CY
High-range water reducer	30 oz/CY
Accelerator	225 oz/CY

Table 15. CLSM Aggregate Specific Gravities and Absorptions

Aggregate	Apparent SG	Bulk SG (dry)	Bulk SG (SSD)	Absorption (%)
Ohio River Sand (Control)	2.645	2.583	2.607	0.93
Manufactured Limestone Sand	2.676	2.588	2.621	1.27
Limestone Screenings	2.760	2.676	2.708	1.11
Crushed Sandstone	2.658	2.611	2.628	0.65





Effects of Substitution of Other Fine Aggregates in ZOOM!

Table 16 shows a comparison of the critical fine aggregate properties for ZOOM! CLSM. Table 17 shows the plastic properties of the ZOOM! CLSM mixtures with other fine aggregates substituted for the Ohio River sand control aggregate. The properties of the other fine aggregates selected are compared with properties of the control aggregate. The following paragraphs describe the effects of the properties of other fine aggregates on properties of ZOOM! CLSM.

Table 16. ZOOM! CLSM Aggregate Property Summary

Aggregate	% Passing #200	FM	Cu	T 304 U _s	T 304 U _m	T 304 U _r
Ohio River Sand (Control)	0.4	2.64	2.10	39.92	42.97	40.19
Manufactured Limestone Sand	5.1	3.10	5.93	41.41	45.43	39.22
Limestone Screenings	15.0	3.25	29.33	48.61	52.38	40.52
Crushed Sandstone	2.6	2.11	2.56	42.90	47.27	43.67

Table 17. ZOOM! CLSM Plastic Properties for other Aggregates using the Control Aggregate Mixture Proportions

Property	Manufactured Limestone Sand	Limestone Screenings	Crushed Sandstone
Flow (inches)	9.75	Shear (No Flow)	6.5
Bleed Time (min)	No Bleeding	No Bleeding	No Bleeding
Shrinkage	No Shrinkage	No Shrinkage	No Shrinkage
Air Content (%)	31.1	16.6	30.4
Unit Weight (pcf)	102.1	121.76	99.2
Problems	None	Flow & Air	Flow
Possible Solutions	None Required	Increase air volume & paste fluidity	Make paste more fluid

A. Manufactured Limestone Sand

Previous research at TTU (10) indicated that a more angular aggregate (as indicated by AASHTO T 304-96 Method B U_m = 45.43) might entrain more air than the control aggregate (U_m = 42.97). Air content increased from 25.7 percent for the control aggregate to 31.1 percent for manufactured limestone sand aggregate ZOOM!. Flow increased from 8.5 inches to 9.75 inches due to the increased air content. Bleeding was reduced by a denser gradation and higher fines content (C_u = 5.93, 5.1%) compared to the control aggregate (C_u = 2.10, 0.4%).

B. Limestone Screenings

Two conflicting factors effected air content of the ZOOM! limestone screenings mixture. As previously stated, more angular aggregates increase air entrainment. Limestone screenings are much more angular than the control aggregate as indicated by both AASHTO T 304 Methods A and B ($U_s = 48.61$ and $U_m = 52.38$ for limestone screenings vs. $U_s = 39.92$ and $U_m = 42.97$ for the control aggregate). However, the dominant factor was fines content (15% vs. 0.4%) reducing the air content from 25.7 percent to 16.6 percent.

Several factors reduced the flow of the limestone screenings ZOOM! mixture to zero. First, the particle shape ($U_s = 48.61$ and $U_m = 52.38$) of the limestone screenings approached a flat and elongate condition. ACI 221-96 (27) sited the work of Gray and Bell (28) who recommended a maximum U_m of 53 percent to avoid flat and elongate conditions. Second, the high fines content (15% vs. 0.4%) was the most important factor in reducing flow. Third, the denser gradation ($C_u = 29.33$ vs. $C_u = 2.10$ for the control aggregate) made obtaining adequate flow more difficult. Finally, and perhaps least importantly, a higher FM (3.25) indicates a much coarser gradation than ORS (FM = 2.64). Coarser particles are harder to mobilize. Bleeding was not a problem due to the high fines content and denser gradation.

C. Crushed Sandstone

Air content rose from 25.7 to 30.4 percent due to more angular aggregate particles $(U_m=47.27 \text{ for crushed sandstone vs. } U_m=42.97 \text{ for the control aggregate})$. Flow dropped from 8.5 inches to 6.5 inches. The previously mentioned angularity of the

crushed sandstone particles compared to the control aggregate was certainly a factor. In addition, the crushed sandstone had a much finer gradation as indicated by the comparison of fineness modulii (FM = 2.11 for crushed sandstone compared to FM = 2.64 for the control aggregate). The finer aggregate required more paste to coat and mobilize the particles.

Bleeding did not occur with the initial substitution of crushed sandstone for the control aggregate. However, flow concerns required more paste to mobilize aggregate. Unfortunately the gradation ($U_r = 43.67$) is much more open than the control aggregate gradation ($U_r = 40.19$), this would lead to bleeding problems after mixture proportion adjustment. Further, plastic cohesion problems resulted from 58.9 percent of aggregate passing the Number 30 Sieve and being retained on the Number 50 Sieve.

ZOOM! Mixture Proportion Adjustments for Other Fine Aggregates

Adjustments were not required for the limestone manufactured sand ZOOM! CLSM. The adjustments and revised proportions for the limestone screenings and crushed sandstone mixtures are shown in Table 18. Plastic properties for the adjusted mixture proportion limestone screenings and crushed sandstone mixtures are shown in Table 19. The research team was not able to satisfy both flow and bleeding requirements for the crushed sandstone ZOOM! CLSM. The research team was able to raise the flow to 7.25 inches without bleeding by increasing the high-range water reducer.

Table 18. ZOOM! CLSM Adjusted Mixture Proportions for Non-control Aggregates

Component	Limestone Screenings	Crushed Sandstone
Type 1 Portland cement	350 lbs/CY Control + 50 lbs/CY	300 lbs/CY
Water	375 lbs/CY Control + 58 lbs/CY	317 lbs/CY
Fine Aggregate (SSD)	2335 lbs/CY	2460 lbs/CY
Air-entraining agent	105 oz/CY Control + 35 oz/CY	70 oz/CY
High-range water reducer	45 oz/CY Control + 15 oz/CY	91 oz/CY Control + 61 oz/CY
Accelerator	225 oz/CY	225 oz/CY

Table 19. ZOOM! CLSM Plastic Properties for other Aggregates using the Adjusted Mixture Proportions

Property	Limestone Screenings	Crushed Sandstone	Requirement
Flow (inches)	9.50	7.25	8.75 minimum
Bleed Time (min)	No Bleeding	No Bleeding	Little or no bleeding
Shrinkage	No Shrinkage	No Shrinkage	Little or no shrinkage
Air Content (%)	22.0	26.4	20 to 30 preferred
Unit Weight (pcf)	110.6	104.8	No requirement
Meet Requirements?	Yes	No, low flow	

ZOOM! Field Demonstration Results and Analysis

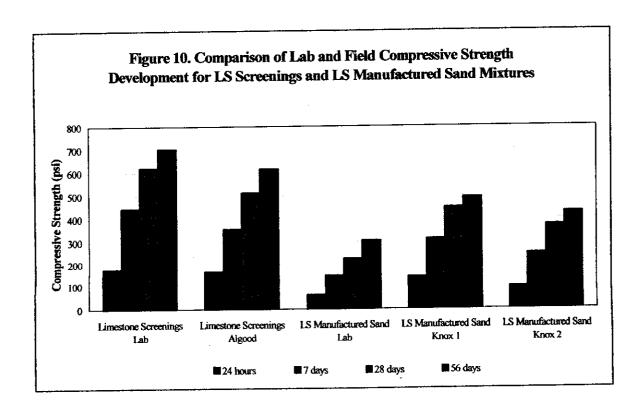
Field demonstrations of ZOOM! CLSM were held in Nashville, Knoxville, and Algood, Tennessee using fine aggregate commonly used for PCC in the area or limestone screenings. Each field demonstration consisted of one or more trench (approximately 3 feet wide, 3.5 feet deep and 9 feet long) placements using the local fine aggregate(s). Testing of the ZOOM! CLSM was conducted at each location and currently available information was distributed to government and industry personnel present.

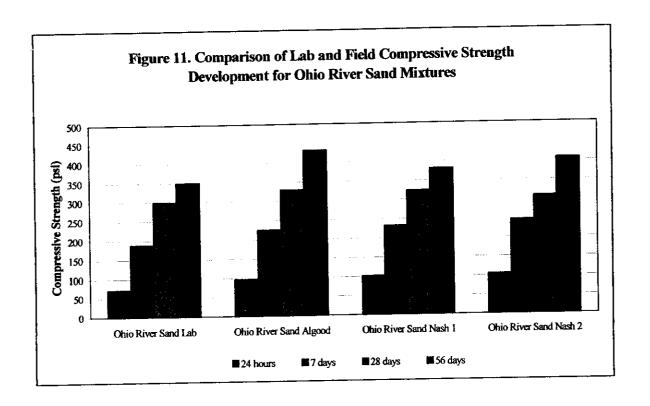
Figures 10 and 11 show comparisons of compressive strength development for field demonstrations and laboratory ZOOM! CLSM mixtures. Figures 12, 13, and 14 show comparison of flow, air content and time to pass the ball drop test, respectively. Compressive strength specimens were not fabricated at Irving Materials Inc. (IMI) Nashville. Ball drop test data is not available for the limestone screenings ZOOM! CLSM at Algood due to excessive water in the trench precluding ball drop testing.

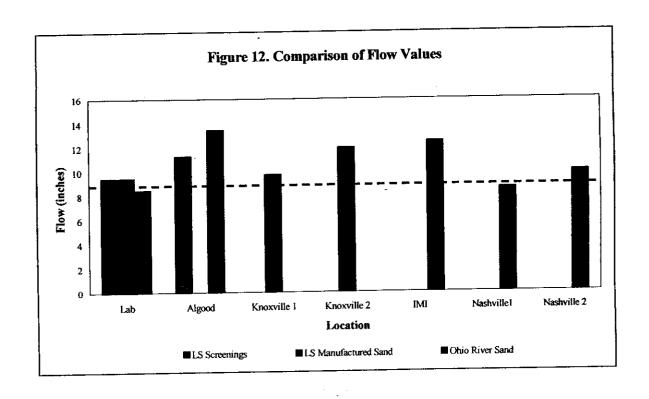
ZOOM! CLSM met compressive strength development and time of set performance criteria at every field demonstration. However, ZOOM! CLSM made with the Ohio River sand control aggregate failed to achieve the desired flow in the lab and for Nashville Number 1 Trench. In each case, the flow was greater than 8 inches but less than 8.75 inches. Limestone manufactured sand ZOOM! CLSM mixtures failed to fall within the desired air content range in the lab and for Knoxville Number 1 Trench. Neither case adversely affected the other mixture properties enough to cause a failure in compressive strength, set time, or flow.

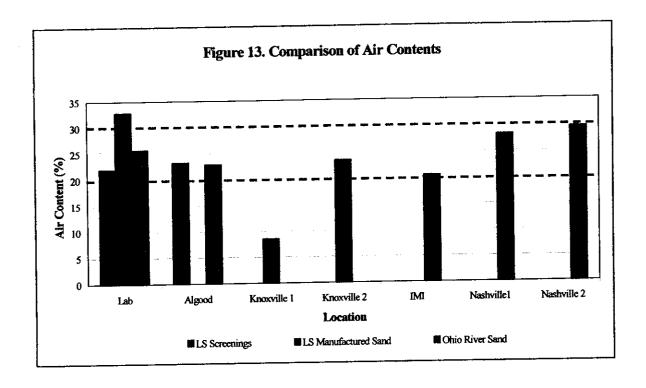
The effect of fine aggregate type on time of set and compressive strength appeared to be indirect. ZOOM! temperature, air content, PC content, and accelerator dosage appeared to have the greatest effect on set time and compressive strength development. Different types of fine aggregates required different PC and air contents to achieve the desired flow characteristics. The research team suspected that the more rapid compressive strength development of limestone screenings was partially due to acceleration of hydration by the limestone fines; however, the limestone screenings mixture had the highest PC content. Therefore, insufficient data was available in this study to make a determination.

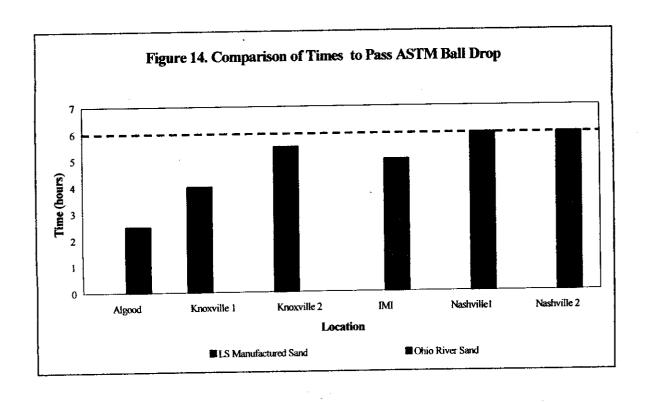
The effect of average air temperature divided by air content on the time to pass the ball drop test is shown in Figure 15. The data for Knoxville Trench 1 is not included in Figure 15 due to the unusually low air content of 8.5 percent. The coefficient of determination was 0.6185 indicating a possible relationship. However, only five data points were available for the correlation. Further, ZOOM! CLSM mixture temperature over time would have been superior to average air temperature, unfortunately that data was not available. Finally, two key factors for time to pass ball drop did not vary in the available data, Portland cement content and accelerator dosage.

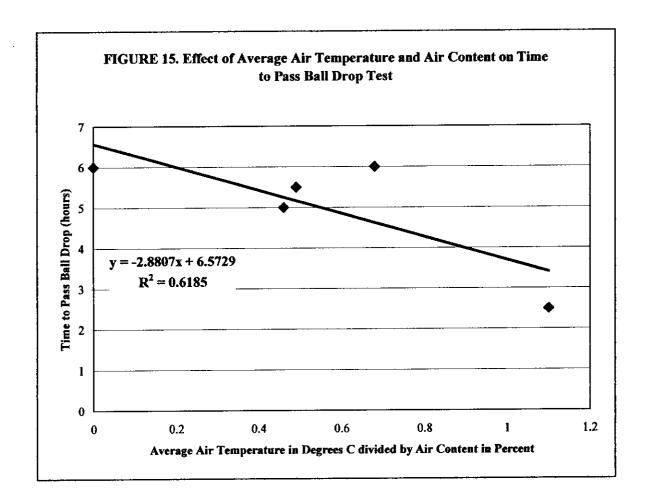












ZOOM! Conclusions

The following conclusions can be drawn from the limited data available in this study:

- A high flow, rapid-set, non-excavatable CLSM for applications where time was a critical factor can be produced with a wide variety of Tennessee fine aggregates.
- Fine aggregate properties such as gradation and angularity dictate mixture proportions required to achieve flow, air content, and bleeding characteristics of CLSM.
- 3) Average air temperature was inversely proportional to time of suitability for load application.
- CLSM air content was directly proportional to time of suitability for load application.

ZOOM! Technology Transfer

Technology transfer was accomplished by preparing a minimum of 1000 copies of a CD-ROM containing the project results and a model specification for ZOOM!. The CD-ROMs were distributed by holding four seminars (Nashville 7/16/03 and Knoxville 7/17/03 in Tennessee and Frankfort 8/13/03 and Owensboro 8/14/03 in Kentucky) for ready mix producers, municipal and state specifying officials. The CD-ROMs were also to be distributed by TRMCA and KRMCA. The CD-ROMs and associated seminars were funded by KRMCA. Additional technology transfer occurred through a presentation entitled "Effect of Fine Aggregate Type on CLSM Properties" at the International Center

for Aggregate Research 11th Annual Symposium on Aggregates: Asphalt Concrete, Portland Cement Concrete, Bases, and Fines in Austin, Texas in April 2003. An accompanying paper was published in the ICAR symposium proceedings. Further technology transfer is planned through a presentation at the ASTM Symposium on Innovations in Controlled Low-Strength Material (Flowable Fill) on June 19, 2003 in Denver, Colorado. A paper entitled "ZOOM!" was submitted for possible publication in ASTM STP 1459 (at the discretion of the ASTM Committee on Publications).

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ZOOM! Disclaimer

The opinions, findings, and conclusions expressed here are those of the authors and not necessarily those of the Tennessee Department of Transportation or the Tennessee Ready Mixed Concrete Association.

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APPENDIX A

Table A1. 7-Day EFF Compressive Strength Data

	Cor	npressive Str	ength	Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
	50	3.98		7.60	130.6		
1 KTC 30/300	67	5.33	4.66	7.71	132.6	131.6	
	77	6.13		7.81	134.3		
Mixture 2 30/370	66	5.25	5.60	7.73	132.9	133.8	
	68	5.41		7.80	134.2		
	89	7.08		7.56	130.0		
Mixture 3 30/440	97	7.72	7.37	7.54	129.6	129.4	
	92	7.32		7.49	128.8		
	107	8.51		7.68	132.0		
Mixture 4 45/300	122	9.71	8.89	7.72	132.6	131.9	
· ·	106	8.44]	7.63	131.2		
	88	7.00	7.98	7.63	131.1	130.6	
5 TRMCA 45/370	110	8.75		7.57	130.0		
	103	8.20		7.60	130.6		
	211	16.79		7.69	132.2		
Mixture 6 45/440	228	18.14	16.55	7.66	131.7	132.1	
	185	14.72		7.70	132.0 132.6 131.2 131.1 130.0 130.6 132.2		
	115	9.15		7.49	128.8		
Mixture 7 60/300	133	10.58	10.00	7.46	128.2	128.7	
	129	10.27		7.50	129.0		
	221	17.59		7.61	130.7		
Mixture 8 60/370	209	16.63	17.08	7.53	129.4	130.1	
	214	17.03		7.57	130.1		
	282	22.44		7.61	130.7		
9 TTU CAP 60/440	392	31.19	27.64	7.64	131.3	131.0	
	368	29.28		7.63	131.1		
10 45/270	264	21.01		7.41	127.3		
Mixture 10 45/370	284	22.60	21.80	7.43	127.7	127.5	
High Carbon Ash	225	17.90		7.42	127.5		

⁻⁻ Sample did not survive de-molding

Table A1. (Continued)

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
11 50 5	335	26.66		7.43	127.6	
Mixture 11 60/370	318	25.31	26.87	7.38	126.9	127.4
High Carbon Ash	360	28.65		7.43	127.7	
	134	10.66		7.46	128.2	
Mixture 12 45/440	146	11.62	11.86	7.45	128.1	128.5
High Carbon Ash	167	13.29		7.52	129.3	
	329	26.18		7.49	128.7	
Mixture 13 60/440	400	31.83	27.59	7.32	125.8	127.7
High Carbon Ash	311	24.75		7.48	128.6	
	282	22.44		7.55	129.7	
Mixture 14 45/510	321	25.54	23.13	7.44	127.8	128.5
High Carbon Ash	269	21.41	1	7.45	128.1	
	536	42.65		7.36	126.5	
Mixture 15 60/510	605	48.14	47.03	7.25	124.6	125.0
High Carbon Ash	632	50.29	1	7.22	124.1	
16 Manufactured	252	20.05		7.49	128.7	
Limestone Sand	285	22.68	20.40	7.43	127.7	128.3
45/370	232	18.46		7.48	128.6	
17 Crushed	175	13.93		7.20	123.7	
Sandstone Sand	146	11.62	12.77	7.37	126.6	125.7
45/370	148	11.78		7.37	126.8	
1031	182	14.48		7.21	123.9]
18 Masonry Sand	283	22.52	18.50	7.29	125.3	124.8
45/370	325	25.86		7.28	125.1	
	285	22.68		7.75	133.2	
19 Limestone	293	23.32	22.94	7.71	132.6	132.7
Screenings 45/370	287	22.84		7.69	132.2	

Table A1. (Continued)

	Compressive Strength			Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	255	20.29		7.40	127.2	
22 TDOT 100/250	239	19.02	20.96	7.35	126.3	126.3
	296	23.55		7.29	125.2	
	148	11.78		5.55	95.4	94.9
23 MBT MB AE 90	126	10.03	10.45	5.50	94.5	
	120	9.55		5.51	94.7	
	386	30.72		6.11	105.0	
24 W.R. Grace	406	32.31	31.59	6.10	104.9	104.5
Darafill	399	31.75		6.03	103.6	
25 MBT Rheofill	145	11.54	11.14	5.84	100.4	
Limestone	138	10.98		5.87	100.8	100.4
Manufactured Sand	137	10.90	<u> </u>	5.81	99.9	

Table A2. 28-Day EFF Compressive Strength Data

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
1 KTC 30/300	74	5.89		7.42	127.5	
	67	5.33	5.73	7.53	129.4	128.2
	75	5.97		7.44	127.8	
	115	9.15		7.36	126.4	
Mixture 2 30/370	142	11.30	10.24	7.75	133.1	129.9
	129	10.27		7.58	130.2	
	122	9.71		7.36	126.6	
Mixture 3 30/440	168	13.37	12.60	7.45	128.1	127.7
	185	14.72		7.47	128.4	
	241	19.18		7.56	129.9	
Mixture 4 45/300	263	20.93	19.60	7.57	130.2	129.9
:	235	18.70		7.54	129.6	
	187	14.88		7.57	130.2	
5 TRMCA 45/370	222	17.67	16.74	7.59	130.4	130.3
·	222	17.67		7.58	130.3	
	248	19.74		7.57	130.1	
Mixture 6 45/440	226	17.98	19.81	7.56	129.9	129.8
	273	21.72		7.53	129.4	
	230	18.30		7.42	127.5	
Mixture 7 60/300	216	17.19	17.32	7.45	128.1	127.6
	207	16.47		7.41	127.3	
	353	28.09		7.47	128.5	<u> </u>
Mixture 8 60/370	319	25.39	26.87	7.47	128.4	128.3
	341	27.14		7.45	128.0	
	564	44.88		7.65	131.4	1
9 TTU CAP 60/440	433	34.46	39.55	7.70	132.3	131.4
	494	39.31		7.60	130.6	
N 10 45/270	442	35.17		7.45	128.1	
Mixture 10 45/370	489	38.91	34.09	7.42	127.5	127.8
High Carbon Ash	354	28.17		7.43	127.7	

Table A2. (Continued)

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	525	41.78		7.45	128.1	
Mixture 11 60/370	552	43.93	40.56	7.46	128.1	127.9
High Carbon Ash	452	35.97		7.42	127.6	
	219	17.43		7.43	127.7	
Mixture 12 45/440	265	21.09	19.84	7.47	128.4	128.5
High Carbon Ash	264	21.01		7.54	129.5	
	359	28.57		7.44	127.8	
Mixture 13 60/440	299	23.79	30.72	7.42	127.5	127.1
High Carbon Ash	500	39.79		7.33	126.0	
	366	29.13		7.45	128.0	
Mixture 14 45/510	365	29.05	28.75	7.42	127.6	128.6
High Carbon Ash	353	28.09]	7.57	130.1	
	811	64.54		7.24	124.5]
Mixture 15 60/510	694	55.23	58.65	7.40	127.2	126.5
High Carbon Ash	706	56.18		7.44	127.8	
16 Manufactured	628	49.97		7.54	129.6	
Limestone Sand	603	47.99	48.60	7.56	129.9	130.1
45/370	601	47.83		7.60	130.7	
17 Crushed	314	24.99		7.30	125.5	_
Sandstone Sand	298	23.71	24.35	7.21	123.9	123.9
45/370	275	21.88		7.12	122.4	
40.14	529	42.10		7.32	125.8	1
18 Masonry Sand	669	53.24	47.67	7.24	124.4	125.1
45/370	415	33.02		7.27	125.0	
	741	58.97		7.73	132.9	
19 Limestone	690	54.91	57.80	7.79	133.9	133.1
Screenings 45/370	748	59.52		7.70	132.3	

Table A2. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	419	33.34		7.32	125.8	·
22 TDOT 100/250	500	39.79	40.08	7.33	125.9	127.3
	592	47.11		7.56	130.0	
	197	15.68		5.37	92.4	
23 MBT MB AE 90			15.68			92.4
			1			
	368	29.28		5.92	101.8	
24 W.R. Grace	524	41.70	35.49	6.00	103.2	102.6
Darafill	446	35.49		5.98	102.9	
25 MBT Rheofill	218	17.35		5.90	101.4	
Limestone	213	16.95	16.68	5.93	101.8	100.7
Manufactured Sand	198	15.76		5.76	99.0	

⁻ Sample did not survive de-molding

Table A3. 63-Day EFF Compressive Strength Data

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	95	7.56		7.31	125.7	
1 KTC 30/300	96	7.64	7.60	7.47	128.4	127.7
	100	7.96		7.51	129.1	
	137	10.90	-	7.32	125.8	
Mixture 2 30/370	148	11.78	11.88	7.32	125.8	127.0
	163	12.97		7.52	129.3	
	216	17.19		7.04	121.0	
Mixture 3 30/440	269	21.41	19.71	7.33	126.0	123.8
	258	20.53		7.23	124.3	
	262	20.85		7.31	125.7	
Mixture 4 45/300	275	21.88	23.95	7.41	127.4	126.8
	366	29.13		7.41	127.4	
	264	21.01		7.15	122.9	
5 TRMCA 45/370	199	15.84	18.20	7.41	127.4	125.3
	223	17.75		7.31	125.7	
	384	30.56		7.32	125.8	
Mixture 6 45/440	488	38.83	37.06	7.68	131.9	130.2
:	525	41.78		7.73	132.9	
	530	42.18		7.31	125.7	
Mixture 7 60/300	391	31.11	37.67	7.40	127.2	126.2
	499	39.71		7.32	125.8	
	676	53.79		7.23	124.2	
Mixture 8 60/370	666	53.00	51.73	7.30	125.4	125.2
	608	48.38		7.32	125.9	
	795	63.26		7.36	126.5	
9 TTU CAP 60/440	1155	91.91	77.96	7.44	127.8	127.3
	989	78.70		7.41	127.4	
Mixture 10 45/370	560	44.56		7.35	126.3	
	436	34.70	44.88	7.32	125.8	126.6
High Carbon Ash	696	55.39		7.42	127.6	

Table A3. (Continued)

	Con	pressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	659	52.44		7.32	125.7	
Mixture 11 60/370	753	59.92	54.51	7.33	125.9	125.6
High Carbon Ash	643	51.17		7.27	125.0	
	313	24.91		7.44	128.0	
Mixture 12 45/440	289	23.00	23.08	7.40	127.3	127.2
High Carbon Ash	268	21.33		7.35	126.4	
	596	47.43		7.49	128.7	j
Mixture 13 60/440	601	47.83	46.63	7.44	127.9	127.1
High Carbon Ash	561	44.64]	7.26	124.8	
	452	35.97		7.23	124.3	
Mixture 14 45/510	432	34.38	35.17	7.34	126.2	125.3
High Carbon Ash						
	873	69.47		6.98	120.0	121.4
Mixture 15 60/510	1011	80.45	76.42	7.09	121.8	
High Carbon Ash	997	79.34		7.12	122.3	
16 Manufactured	886	70.51		7.55	129.8	
Limestone Sand	906	72.10	64.33	7.50	128.9	129.9
45/370	633	50.37		7.63	131.1	
17 Crushed	724	57.61		7.24	124.5]
Sandstone Sand	579	46.08	51.84	6.86	118.0	122.1
45/370	591	47.03		7.20	123.7	
10.16 0 1	480	38.20		6.86	117.8	
18 Masonry Sand	1045	83.16	60.68	7.38	126.9	123.1
45/370	658	52.36		7.25	124.7	
10.7.	995	79.18		7.66	131.7	1
19 Limestone	1087	86.50	82.71	7.61	130.7	131.1
Screenings 45/370	1036	82.44		7.61	130.7	

⁻⁻ Sample did not survive de-molding

Table A3. (Continued)

	Cor	npressive Str	ength	Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
	703	55.94		7.17	123.2		
22 TDOT 100/250	863	68.68	68.38	7.33	126.0	125.3	
	1012	80.53	1	7.37	126.7		
	225	17.90		5.18	89.0		
23 MBT MB AE 90	247	19.66	18.75	5.45	93.7	92.2	
·	235	18.70	_	5.47	94.0		
24 W.D. C	526	41.86		5.94	102.1		
24 W.R. Grace	618	49.18	45.44	6.04	103.8	102.9	
Darafill	569	45.28		5.99	102.9		
25 MBT Rheofill	238	18.94		5.71	98.1		
Limestone	227	18.06	18.09	5.81	99.8	98.4	
Manufactured Sand	217	17.27		5.66	97.3		

Table A4. 98-Day EFF Compressive Strength Data

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	132	10.50		7.11	122.2	
1 KTC 30/300	113	8.99	8.86	7.17	123.3	124.4
	89	7.08		7.42	127.6	
	221	17.59		7.47	128.4	
Mixture 2 30/370	159	12.65	14.93	7.35	126.4	126.7
	183	14.56	1	7.29	125.2	
	219	17.43		7.08	121.6	
Mixture 3 30/440	299	23.79	20.16	7.21	123.9	122.9
	242	19.26]	7.17	123.2	
	482	38.36		7.34	126.1	
Mixture 4 45/300	510	40.58	39.76	7.37	126.7	126.4
	507	40.35		7.35	126.4	
	271	21.57		7.14	122.7	
5 TRMCA 45/370	229	18.22	20.56	7.16	123.0	123.0
	275	21.88		7.17	123.2	
	441	35.09		7.13	122.6	122.7
Mixture 6 45/440	619	49.26	38.17	7.24	124.5	
	379	30.16		7.04	121.1	
	727	57.85		7.16	123.0	
Mixture 7 60/300	668	53.16	56.00	7.25	124.5	123.9
	716	56.98		7.22	124.1	
	685	54.51		7.20	123.8	·
Mixture 8 60/370	809	64.38	60.16	7.19	123.6	123.6
	774	61.59		7.18	123.4	
9 TTU CAP 60/440	1591	126.61		7.26	124.8	
	1231	97.96	107.83	7.33	126.1	124.3
	1243	98.91		7.10	122.0	
) C	519	41.30		7.22	124.1	
Mixture 10 45/370	409	32.55	41.14	7.12	122.3	123.6
High Carbon Ash	623	49.58		7.24	124.4	

Table A4. (Continued)

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
_	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1116	88.81		7.13	122.5	
Mixture 11 60/370	845	67.24	80.82	6.99	120.2	120.0
High Carbon Ash	1086	86.42		6.82	117.2	
	278	22.12		7.21	123.8	
Mixture 12 45/440	334	26.58	23.93	7.41	127.3	125.0
High Carbon Ash	290	23.08		7.21	123.9	
	583	46.39		7.25	124.5	[
Mixture 13 60/440	525	41.78	42.65	7.22	124.1	124.4
High Carbon Ash	500	39.79]	7.24	124.5	
	552	43.93		7.12	122.4	
Mixture 14 45/510	447	35.57	37.80	7.25	124.7	123.4
High Carbon Ash	426	33.90	1	7.17	123.2	
	958	76.24	88.60	7.17	123.3	122.3
Mixture 15 60/510	1135	90.32		6.99	120.2	
High Carbon Ash	1247	99.23		7.19	123.5	
16 Manufactured	998	79.42		7.44	127.9	
Limestone Sand	1120	89.13	85.25	7.37	126.8	127.1
45/370	1096	87.22		7.38	126.8	
17 Crushed	958	76.24		7.04	121.0	
Sandstone Sand	1019	81.09	78.66	7.25	124.5	122.1
45/370	689	54.83		7.03	120.8	
4036 0 1	697	55.47		6.74	115.8	1
18 Masonry Sand 45/370	1111	88.41	71.94	7.23	124.3	120.6
	1149	91.43		7.08	121.7	
	1169	93.03		7.55	129.8	
19 Limestone	1163	92.55	91.59	7.62	131.0	129.5
Screenings 45/370	1121	89.21		7.44	127.8	

Table A4. (Continued)

	Cor	npressive Str	ength	Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
	1301	103.53		7.22	124.0		
22 TDOT 100/250	1574	125.25	97.54	7.23	124.3	123.2	
	802	63.82		7.06	121.3		
	281	22.36		5.44	93.6		
23 MBT MB AE 90	295	23.48	22.28	5.55	95.4	93.8	
	264	21.01		5 <i>.</i> 37	92.3		
24 W.D. C	671	53.40		5.84	100.4		
24 W.R. Grace	544	43.29	50.35	6.06	104.1	102.9	
Darafill	683	54.35		6.07	104.3		
25 MBT Rheofill	236	18.78		5.78	99.3	-	
Limestone	248	19.74	19.68	5.59	96.2	97.6	
Manufactured Sand	258	20.53		5.66	97.3	!	

Table A5. 140-Day EFF Compressive Strength Data

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	99	7.88		7.06	121.3	
1 KTC 30/300	103	8.20	8.97	6.84	117.6	121.4
	136	10.82		7.29	125.3	
	189	15.04		7.35	126.3	
Mixture 2 30/370	114	9.07	13.00	6.87	118.1	120.5
	187	14.88		6.82	117.1	
	263	20.93		7.10	122.0	1
Mixture 3 30/440	228	18.14	18.94	6.75	116.1	118.8
	223	17.75		6.89	118.5	
	544	43.29		6.84	117.6	
Mixture 4 45/300	455	36.21	36.58	7.05	121.2	118.7
	380	30.24	1	6.82	117.3	
	364	28.97		7.03	120.8	
5 TRMCA 45/370	272	21.65	24.43	6.82	117.2	119.2
	285	22.68		6.96	119.7	
	727	57.85		7.16	123.1	
Mixture 6 45/440	736	58.57	55.76	7.17	123.3	124.2
	639	50.85		7.35	126.3	
	739	58.81		7.14	122.7	
Mixture 7 60/300	789	62.79	66.26	7.11	122.2	123.2
	970	77.19		7.25	124.6	
	920	73.21		7.12	122.4	
Mixture 8 60/370	830	66.05	69.26	7.18	123.5	122.5
	861	68.52		7.09	121.8	
	1143	90.96		7.06	121.4	
9 TTU CAP 60/440	1613	128.36	110.80	7.26	124.8	123.9
	1421	113.08		7.31	125.7	
10 45/270	679	54.03		7.20	123.8]
Mixture 10 45/370	1067	84.91	67.77	6.78	116.5	119.0
High Carbon Ash	809	64.38		6.79	116.7	

Table A5. (Continued)

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
11 (0/270	1384	110.14	•	6.87	118.1	
Mixture 11 60/370	1386	110.29	108.38	6.97	119.8	120.3
High Carbon Ash	1316	104.72		7.15	123.0	
10 45/440	288	22.92		7.18	123.5	1
Mixture 12 45/440	286	22.76	22.71	7.11	122.3	122.3
High Carbon Ash	282	22.44		7.04	121.1	
12 (0/440	838	66.69	***	6.85	117.8	
Mixture 13 60/440	726	57.77	62.95	7.21	124.0	121.5
High Carbon Ash	809	64.38		7.15	122.8	
) C 4 14 45/510	647	51.49		7.04	121.0	
Mixture 14 45/510	644	51.25	53.56	7.07	121.6	121.2
High Carbon Ash	728	57.93		7.04	121.1	
15 (0/510	1502	119.53		7.06	121.3	
Mixture 15 60/510	1385	110.21	101.65	7.10	122.1	121.8
High Carbon Ash	945	75.20		7.09	121.9	
16 Manufactured	1086	86.42		7.37	126.6	
Limestone Sand	943	75.04	80.98	7.41	127.3	127.6
45/370	1024	81.49		7.50	129.0	
17 Crushed	1128	89.76		6.64	114.1	
Sandstone Sand	861	68.52	79.14	6.98	119.9	116.5
45/370	1153	91.75		6.72	115.4	
10 Maganga Cand	1270	101.06		7.07	121.5	
18 Masonry Sand 45/370	1579	125.65	113.36	6.74	115.9	120.2
43/3/0	1354	107.75		7.17	123.2	
19 Limestone	1155	91.91		7.53	129.4	129.0
,	1184	94.22	98.97	7.60	130.6	
Screenings 45/370	1392	110.77		7.38	126.9	

Table A5. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1843	146.66		6.99	120.1	
22 TDOT 100/250	1951	155.26	141.14	7.00	120.3	121.3
22 11501 100/250	1527	121.51		7.18	123.4	
	294	23.40		5.28	90.8	89.4
23 MBT MB AE 90	342	27.22	23.77	5.14	88.3	
	260	20.69		5.18	89.1	
	595	47.35		5.90	101.4	
24 W.R. Grace	676	53.79	51.01	5.97	102.7	101.7
Darafill	652	51.88		5.87	101.0	
25 MBT Rheofill	300	23.87		5.37	92.3	
Limestone	295	23.48	23.63	5.52	94.8	93.9
Manufactured Sand	296	23.55		5.50	94.5	

Table A6. 182-Day EFF Compressive Strength Data

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	140	11.14	.,,	7.20	123.8	
1 KTC 30/300	99	7.88	9.47	6.87	118.1	118.9
	118	9.39	:	6.67	114.7	
	206	16.39		7.15	122.9	
Mixture 2 30/370	192	15.28	15.09	7.53	129.5	127.7
	171	13.61		7.61	130.8	
	242	19.26		6.82	117.2	
Mixture 3 30/440	293	23.32	21.11	6.86	118.0	118.8
	261	20.77		7.05	121.1	
	502	39.95		7.05	121.1	121.5
Mixture 4 45/300	723	57.53	45.31	7.25	124.6	
	483	38.44		6.90	118.6	
	465	37.00		6.87	118.1	
5 TRMCA 45/370	396	31.51	34.88	7.11	122.1	119.1
	454	36.13		6.81	117.1	
	746	59.36	•	7.27	124.9	
Mixture 6 45/440	824	65.57	65.36	7.13	122.5	125.0
	894	71.14		7.42	127.5	
	941	74.88		6.77	116.3	
Mixture 7 60/300	1000	79.58	74.01	6.78	116.6	118.1
-	849	67.56		7.06	121.4	
	1072	85.31		6.86	118.0	
Mixture 8 60/370	996	79.26	80.51	6.84	117.5	119.3
	967	76.95		7.13	122.5	
	1859	147.93		7.10	122.0	
9 TTU CAP 60/440	2093	166.56	145.10	7.08	121.7	122.5
	1518	120.80		7.19	123.7	
Mixture 10 45/370	780	62.07		6.99	120.1	
High Carbon Ash	1233	98.12	82.73	7.06	121.4	121.2
riigii Carbon Ash	1106	88.01		7.11	122.1	

Table A6. (Continued)

	Con	pressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
2.2	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1431	113.88		6.78	116.6	
Mixture 11 60/370	1558	123.98	114.46	6.79	116.6	117.9
High Carbon Ash	1326	105.52		7.00	120.4	
	199	15.84		7.02	120.7	
Mixture 12 45/440	208	16.55	20.64	6.96	119.7	119.4
High Carbon Ash	371	29.52	- 	6.86	117.8	
	612	48.70		6.83	117.4	
Mixture 13 60/440	509	40.50	49.55	6.96	119.7	119.6
High Carbon Ash	747	59.44		7.09	121.8	
	634	50.45		6.92	118.9	
Mixture 14 45/510	563	44.80	51.33	6.64	114.1	115.9
High Carbon Ash	738	58.73		6.67	114.6	
	1442	114.75	117.14	7.12	122.4	120.1
Mixture 15 60/510	1526	121.44		7.03	120.8	
High Carbon Ash	1448	115.23		6.81	117.1	
16 Manufactured	1087	86.50		7.31	125.6	
Limestone Sand	996	79.26	83.08	7.19	123.5	123.6
45/370	1049	83.48		7.09	121.8	ļ
17 Crushed	1266	100.75		7.11	122.2	
Sandstone Sand	943	75.04	87.89	6.72	115.5	118.8
45/370	1057	84.11	1	6.91	118.7	
	1281	101.94		6.74	115.9	
18 Masonry Sand 45/370	1361	108.30	105.12	6.59	113.2	115.2
	1214	96.61		6.78	116.5	
	1369	108.94		7.28	125.1	_
19 Limestone	1336	106.32	104.96	7.16	123.1	124.3
Screenings 45/370	1252	99.63]	7.26	124.8	

Table A6. (Continued)

	Cor	npressive Str	ength	Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
	2521	200.61		6.96	119.7		
22 TDOT 100/250	2247	178.81	184.41	7.45	128.0	122.3	
	2184	173.80		6.94	119.3		
	322	25.62		5.18	89.0		
23 MBT MB AE 90	237	18.86	25.39	5.00	86.0	88.7	
	319	25.39		5.31	91.2		
04 W D C	967	76.95		5.71	98.1		
24 W.R. Grace	951	75.68	72.92	5.68	97.7	98.2	
Darafill	831	66.13		5.76	98.9		
25 MBT Rheofill	406	32.31		5.29	90.9		
Limestone	273	21.72	25.65	5.52	94.9	93.2	
Manufactured Sand	288	22.92		5.46	93.9		

Table A7. 238-Day EFF Compressive Strength Data

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	106	8.44		6.69	115.0	
1 KTC 30/300	110	8.75	8.59	6.55	112.6	113.8
	173	13.77		6.93	119.2	
Mixture 2 30/370	172	13.69	13.13	6.91	118.8	118.3
1	150	11.94		6.80	116.9	
	225	17.90		6.61	113.7	
Mixture 3 30/440	251	19.97	20.37	6.49	111.6	116.0
	292	23.24		7.13	122.6	
	470	37.40		6.90	118.5	·
Mixture 4 45/300	383	30.48	42.73	6.90	118.7	118.7
	758	60.32		6.91	118.8	
	475	37.80		6.88	118.2	
5 TRMCA 45/370	407	32.39	38.70	6.84	117.6	118.3
	577	45.92		6.92	118.9	
	643	51.17		7.14	122.7	
Mixture 6 45/440	918	73.05	61.14	7.13	122.6	122.5
	744	59.21		7.12	122.4	1
	1187	94.46		6.70	115.2	
Mixture 7 60/300	1077	85.70	98.60	6.85	117.8	116.7
	1453	115.63		6.82	117.2	
	1033	82.20		6.79	116.6	
Mixture 8 60/370	1090	86.74	85.86	6.87	118.0	117.0
	1114	88.65		6.77	116.3	
	1400	111.41		6.88	118.2	
9 TTU CAP 60/440	1795	142.84	125.89	6.97	119.8	119.6
	1551	123.42		7.03	120.8	
10 45/270	800	63.66		6.72	115.6	
Mixture 10 45/370	990	78.78	74.56	6.66	114.5	115.8
High Carbon Ash	1021	81.25		6.83	117.4	

⁻ Sample did not survive de-molding

Table A7. (Continued)

	Con	pressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
ev and a vota v	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1324	105.36		6.68	114.9	
Mixture 11 60/370	1548	123.19	122.42	6.77	116.4	116.0
High Carbon Ash	1743	138.70		6.79	116.7	
	354	28.17		6.96	119.6	
Mixture 12 45/440	421	33.50	29.79	6.72	115.5	117.2
High Carbon Ash	348	27.69]	6.78	116.6	
	792	63.03		6.63	114.0	
Mixture 13 60/440	918	73.05	67.53	6.92	118.9	115.9
High Carbon Ash	836	66.53		6.68	114.9	
Mixture 14 45/510 High Carbon Ash	899	71.54		6.68	114.8	
	687	54.67	62.36	6.61	113.6	114.7
	765	60.88		6.73	115.7	
	1638	130.35		6.86	117.9	
Mixture 15 60/510	1642	130.67	127.19	6.72	115.5	116.0
High Carbon Ash	1515	120.56] _	6.66	114.4	
16 Manufactured	1142	90.88		7.14	122.7	_
Limestone Sand	1309	104.17	96.71	6.88	118.2	119.7
45/370	1195	95.10	1	6.88	118.3	
17 Crushed	1329	105.76		6.58	113.2 .	<u> </u>
Sandstone Sand	1641	130.59	118.17	6.79	116.8	115.3
45/370	1488	118.41		6.75	116.1	
	1406	111.89		7.15	122.8	
18 Masonry Sand	1295	103.05	107.47	7.08	121.6	119.4
45/370	1311	104.33		6.62	113.7	
	1424	113.32		7.34	126.2	
19 Limestone	1387	110.37	112.20	7.20	123.8	125.7
Screenings 45/370	1419	112.92		7.39	127.1	

Table A7. (Continued)

`	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
22 TDOT 100/250	2079	165.44		6.95	119.4	
	2314	184.14	176.69	6.85	117.7	118.9
	2268	180.48		6.96	119.6	
	431	34.30	38.89	5.22	89.8	88.5
23 MBT MB AE 90	550	43.77		5.03	86.4	
	485	38.60		5.19	89.2	
24 W.D. Cross	1016	80.85		5.47	94.1	94.3
24 W.R. Grace	940	74.80	77.83	5.57	95.7	
Darafill	978	77.83		5.41	93.0	
25 MBT Rheofill	413	32.87		5.37	92.3	
Limestone	541	43.05	36.23	5.23	90.0	91.0
Manufactured Sand	412	32.79		5.28	90.8	

Table A8. 301-Day EFF Compressive Strength Data

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	110	8.75		6.55	112.6	
1 KTC 30/300	72	5.73	8.01	6.52	112.0	112.6
• • • • • • • • • • • • • • • • • • • •	120	9.55		6.59	113.2	
	183	14.56		6.96	119.6	
Mixture 2 30/370	101	8.04	12.04	6.56	112.8	117.3
	170	13.53		6.95	119.5	
	272	21.65	21.94	6.42	110.4	
Mixture 3 30/440	262	20.85		6.54	112.4	112.6
	293	23.32		6.69	114.9	
e (†	585	46.55		6.81	117.1	
Mixture 4 45/300	500	39.79	45.25	6.77	116.3	116.7
	621	49.42		6.78	116.5	
	379	30.16		6.71	115.3	
5 TRMCA 45/370	459	36.53	31.46	6.82	117.2	115.8
	348	27.69		6.69	115.0	
	634	50.45		6.96	119.6	
Mixture 6 45/440	588	46.79	52.26	6.84	117.6	118.7
	748	59.52		6.92	118.9	
	1204	95.81		6.62	113.8	_
Mixture 7 60/300	827	65.81	78.86	6.58	113.1	113.7
	942	74.96	Ī	6.64	114.1	
	1055	83.95		6.64	114.1	4
Mixture 8 60/370	1026	81.65	89.84	6.74	115.9	115.3
	1306	103.93		6.74	115.9	
	1209	96.21		6.94	119.3	
9 TTU CAP 60/440	1454	115.71	136.85	6.90	118.6	119.2
	2496	198.63		6.96	119.6	
Mixture 10 45/370	1036	82.44	86.58	6.68	114.8	115.4
	1336	106.32		6.66	114.5	
High Carbon Ash	892	70.98		6.80	116.9	<u> </u>

Table A8. (Continued)

	Cor	npressive Str	ength	Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
11.60/272	1554	123.66	-	6.66	114.5	114.5	
Mixture 11 60/370	1544	122.87	144.25	6.62	113.8		
High Carbon Ash	2340	186.21		6.70	115.2		
Mixture 12 45/440	362	28.81		6.60	113.4		
Mixture 12 45/440	317		6.76	116.2	114.8		
High Carbon Ash	411	32.71		6.68	114.8		
10.00/440	1194	95.02		6.60	113.4		
Mixture 13 60/440	927	73.77	3.77 83.18	6.58	113.1	113.2	
High Carbon Ash	1015	80.77		6.58	113.1		
Mixture 14 45/510 High Carbon Ash	717	57.06		6.66	114.5		
	721	57.38	59.39	6.68	114.8	114.5	
	801	63.74		6.64	114.1		
15.00510	1457	115.94	131.91	6.62	113.8		
Mixture 15 60/510	1915	152.39		6.52	112.1	113.6	
High Carbon Ash	1601	127.40		6.68	114.8		
16 Manufactured	1141	90.80		6.80	116.9		
Limestone Sand	943	75.04	87.93	7.28	125.1	120.8	
45/370	1231	97.96		7.00	120.3		
17 Crushed	1508	120.00		6.40	110.0		
Sandstone Sand	1654	131.62	125.81	6.48	111.4	110.7	
45/370	1300	103.45		6.44	110.7		
1034 01	1351	107.51		6.62	113.8		
18 Masonry Sand 45/370	1362	108.38	107.95	6.48	111.4	112.2	
	1655	131.70		6.48	111.4		
19 Limestone	1396	111.09		6.98	120.0	118.6	
	1624	129.23	116.85	6.90	118.6		
Screenings 45/370	1385	110.21		6.82	117.2		

Table A8. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load (lbs)	Individual (psi)	Average (psi)	Weight (lbs)	Individual (pcf)	Average (pcf)
22 TDOT 100/250	1711	136.16	168.25	6.94	119.3	
	2482	197.51		6.76	116.2	117.8
	2150	171.09		6.86	117.9	
	431	34.30	37.59	4.94	84.9	
23 MBT MB AE 90	445	35.41		4.88	83.9	85.7
	541	43.05		5.14	88.4	
	992	78.94		5.42	93.2	94.4
24 W.R. Grace	889	70.74	73.69	5.50	94.5	
Darafill	897	71.38		5.56	95.6	
25 MBT Rheofill	399	31.75	41.51	5.46	93.9	91.2
Limestone	561	44.64		5.20	89.4	
Manufactured Sand	605	48.14		5.26	90.4	

Table A9. 364-Day EFF Compressive Strength Data

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	98	7.80		6.42	110.4	109.0
1 KTC 30/300	117	9.31	8.55	6.26	107.6	
	191	15.20		6.88	118.3	
Mixture 2 30/370	184	14.64	15.89	6.74	115.9	116.8
	224	17.83		6.76	116.2	
· · · · · · · · · · · · · · · · · · ·	157	12.49		6.72	115.5	
Mixture 3 30/440	310	24.67	20.27	6.68	114.8	115.6
	297	23.63		6.78	116.5	
	654	52.04		6.72	115.5	
Mixture 4 45/300	445	35.41	42.65	6.68	114.8	115.3
	509	40.50		6.72	115.5	
	466	37.08		6.74	115.9	
5 TRMCA 45/370	468	37.24	40.72	6.82	117.2	118.8
	601	47.83		7.18	123.4	
	776	61.75		6.82	117.2	
Mixture 6 45/440	517	41.14	45.41	6.86	117.9	116.8
	419	33.34		6.70	115.2	
	880	70.03		6.48	. 111.4	
Mixture 7 60/300	1280	101.86	79.52	6.52	112.1	112.0
	838	66.69]	6.54	112.4	
	1001	79.66		6.74	115.9	
Mixture 8 60/370	961	76.47	75.60	6.66	114.5	114.7
	888	70.66		6.62	113.8	
	1361	108.30]	6.90	118.6	
9 TTU CAP 60/440	1276	101.54	101.33	6.94	119.3	118.8
	1183	94.14		6.90	118.6	
Minton 10 45/270	1481	117.85		6.66	114.5	
Mixture 10 45/370	579	46.08	74.22	6.38	109.7	112.2
High Carbon Ash	738	58.73		6.54	112.4	

⁻ Sample did not survive de-molding

Table A9. (Continued)

	Con	pressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	2090	166.32		6.74	115.9	
Mixture 11 60/370	1267	100.82	138.68	6.72	115.5	115.5
High Carbon Ash	1871	148.89		6.70	115.2	
	383	30.48		6.62	113.8	<u> </u>
Mixture 12 45/440	329	26.18	28.33	6.62	113.8	113.8
High Carbon Ash						
_	917	72.97		6.86	117.9	1
Mixture 13 60/440	591	47.03	62.63	6.66	114.5	115.4
High Carbon Ash	853	67.88		6.62	113.8	
Mixture 14 45/510	972	77.35		6.58	113.1	113.3
	861	68.52	67.93	6.52	112.1	
High Carbon Ash	728	57.93		6.68	114.8	
	1425	113.40	136.95	6.52	112.1	_
Mixture 15 60/510	1790	142.44		6.50	111.7	112.3
High Carbon Ash	1948	155.02	Ī	6.58	113.1	
16 Manufactured	1382	109.98		6.64	114.1	
Limestone Sand	1383	110.06	106.42	6.72	115.5	117.7
45/370	1247	99.23	1	7.18	123.4	
17 Crushed	1355	107.83		6.42	110.4	
Sandstone Sand	1323	105.28	106.55	6.34	109.0	111.5
45/370	1587	126.29		6.70	115.2	
	1376	109.50		6.54	112.4	
18 Masonry Sand	1397	111.17	110.33	6.34	109.0	110.9
45/370	1402	111.57		6.48	111.4	
	1779	141.57		6.86	117.9	117.6
19 Limestone	1647	131.06	133.66	6.78	116.5	
Screenings 45/370	1613	128.36		6.88	118.3	

⁻⁻ Sample did not survive de-molding

Table A9. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
22 TDOT 100/250	2550	202.92		6.94	119.3	
	2730	217.25	201.36	6.86	117.9	118.3
	2311	183.90		6.84	117.6	
	418	33.26	35.15	4.90	84.2	84.9
23 MBT MB AE 90	415	33.02		5.00	85.9	
	492	39.15		4.92	84.6	
	993	79.02		5.52	94.9	95.0
24 W.R. Grace	971	77.27	79.13	5.50	94.5	
Darafill	1019	81.09		5.56	95.6	
25 MBT Rheofill	504	40.11		5.14	88.4	
Limestone	574	45.68	43.74	5.26	90.4	89.0
Manufactured Sand	571	45.44		5.14	88.4	

Table A10. 455-Day EFF Compressive Strength Data

`	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	90	7.16		6.30	108.3	
1 KTC 30/300	106	8.44	7.80	6.52	112.1	110.2
					•	
	168	13.37		6.82	117.2	
Mixture 2 30/370	154	12.25	13.74	6.80	116.9	117.6
	196	15.60		6.90	118.6	
	242	19.26		6.60	113.4	
Mixture 3 30/440	197	15.68	19.89	6.80	116.9	113.8
	311	24.75		6.46	111.0	
	503	40.03		6.66	114.5	
Mixture 4 45/300	480	38.20	42.20	6.72	115.5	115.1
	608	48.38		6.70	115.2	
	330	26.26		6.62	113.8	
5 TRMCA 45/370	328	26.10	27.27	6.64	114.1	114.2
	370	29.44		6.68	114.8	
	652	51.88		6.82	117.2	
Mixture 6 45/440	843	67.08	55.55	6.68	114.8	116.2
	599	47.67		6.78	116.5	
	748	59.52		6.56	112.8	
Mixture 7 60/300	554	44.09	59.26	6.40	110.0	111.8
	932	74.17		6.56	112.8	
	878	69.87		6.70	115.2	
Mixture 8 60/370	913	72.65	70.72	6.60	113.4	114.4
	875	69.63		6.66	114.5	
	1495	118.97	·	6.88	118.3	
9 TTU CAP 60/440	1584	126.05	129.39	6.86	117.9	118.5
	1799	143.16		6.94	119.3	
Mixture 10 45/370	1304	103.77		6.60	113.4	
	1308	104.09	94.17	6.56	112.8	113.3
High Carbon Ash	938	74.64		6.62	113.8	

⁻ Sample did not survive de-molding

Table A10. (Continued)

	Con	pressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
•	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	998	79.42		6.64	114.1	
Mixture 11 60/370	718	57.14	72.87	6.62	113.8	114.5
High Carbon Ash	1031	82.04		6.72	115.5	
	301	23.95		6.58	113.1	
Mixture 12 45/440	389	30.96	29.55	6.62	113.8	113.8
High Carbon Ash	424	33.74		6.66	114.5	
	1187	94.46		6.46	111.0	
Mixture 13 60/440	465	37.00	66.66	6.64	114.1	112.6
High Carbon Ash	861	68.52		6.56	112.8	
Mixture 14 45/510 High Carbon Ash	790	62.87		6.46	111.0	112.5
	569	45.28	57.83	6.56	112.8	
	821	65.33		6.62	113.8	
	1590	126.53	123.29	6.48	111.4	111.6
Mixture 15 60/510	1673	133.13		6.46	111.0	
High Carbon Ash	1385	110.21	1	6.54	112.4	
16 Manufactured	910	72.42		6.74	115.9	
Limestone Sand	1126	89.60	87.69	7.14	122.7	117.5
45/370	1270	101.06		6.62	113.8	
17 Crushed	1203	95.73		6,36	109.3	
Sandstone Sand	1531	121.83	108.78	6.52	112.1	112.9
45/370	1349	107.35	1	6.82	117.2	
	1469	116.90		6.52	112.1	
18 Masonry Sand	1186	94.38	105.64	6.38	109.7	110.9
45/370	1257	100.03		6.46	111.0	
	1613	128.36		6.94	119.3	. 118.3
19 Limestone	1526	121.44	120.59	6.84	117.6	
Screenings 45/370	1407	111.97		6.86	117.9	<u> </u>

Table A10. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
22 TDOT 100/250	1937	154.14		6.66	114.5	
	2192	174.43	170.11	6.82	117.2	116.1
	2284	181.75		6.78	116.5]
	598	47.59		5.04	86.6	
23 MBT MB AE 90	405	32.23	34.54	4.98	85.6	86.1
	299	23.79		5.00	85.9	
04 771 D. C.	783	62.31		5.54	95.2	
24 W.R. Grace	903	71.86	65.23	5.40	92.8	93.5
Darafill	773	61.51		5.38	92.5	
25 MBT Rheofill	613	48.78		5.18	89.0	
Limestone	538	42.81	45.54	5.22	89.7	89.4
Manufactured Sand	566	45.04		5.20	89.4	

Table A11. 546-Day EFF Compressive Strength Data

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	81	6.45		6.54	112.3	
1 KTC 30/300	66	5.25	6.76	6.30	108.3	111.2
TRIC SUISC	108	8.59		6.58	113.0	
	127	10.11		6.62	113.7	
Mixture 2 30/370	125	9.95	10.00	6.76	116.2	114.7
WILLIAM DE CONTRACTOR DE CONTR	125	9.95		6.64	114.1	
	211	16.79		6.55	112.6	
Mixture 3 30/440	277	22.04	20.64	6.61	113.6	113.1
	290	23.08		6.58	113.1	
	483	38.44		6.69	114.9	
Mixture 4 45/300	395	31.43	37.37	6.69	115.0	115.2
	531	42.26		6.73	115.8	
	285	22.68		6.72	115.4	
5 TRMCA 45/370	444	35.33	33.13	6.65	114.3	115.0
J 114.1.011	520	41.38		6.71	115.3	
	784	62.39	44.62	6.93	119.2	118.1
Mixture 6 45/440	470	37.40		6.86	117.9	
	428	34.06	1	6.82	117.2	
	1081	86.02		6.57	112.9	
Mixture 7 60/300	852	67.80	66.05	6.60	113.4	112.8
171750010 , 00,000	557	44.32		6.52	112.0	
	1134	90.24		6.65	114.4	
Mixture 8 60/370	863	68.68	80.27	6.73	115.6	114.8
	1029	81.89		6.66	114.5	<u> </u>
· · · · · · · · · · · · · · · · · · ·	1372	109.18		6.83	117.5	
9 TTU CAP 60/440	1109	88.25	89.23	6.96	119.6	119.1
	883	70.27		7.00	120.3	
	1115	88.73		6.63	113.9	
Mixture 10 45/370	696	55.39	79.31	6.64	114.1	114.1
High Carbon Ash	1179	93.82		6.65	114.3	<u> </u>

Table A11. (Continued)

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	859	68.36		6.71	115.3	
Mixture 11 60/370 High Carbon Ash	1290	102.65	90.61	6.68	114.9	114.8
	1267	100.82		6.64	114.1	
	278	22.12		6.64	114.1	
Mixture 12 45/440	307	24.43	23.08	6.60	113.4	114.0
High Carbon Ash	285	22.68		6.66	114.5	
	794	63.18		6.62	113.9	
Mixture 13 60/440	694	55.23	58.86	6.65	114.3	114.5
High Carbon Ash	arbon Ash 731 58.17	6.71	115.3			
	770	61.27		6.58	113.1	
Mixture 14 45/510 High Carbon Ash	835	66.45	59.50	6.59	113.3	113.2
	638	50.77		6.59	113.3	
	1573	125.18	137.11	6.52	112.1	112.1
Mixture 15 60/510	1997	158.92		6.49	111.5	
High Carbon Ash	1599	127.24	·	6.56	112.8	
16 Manufactured	1266	100.75		7.14	122.8	
Limestone Sand	970	77.19	89.87	6.48	111.4	115.8
45/370	1152	91.67]	6.59	113.3	
17 Crushed	1234	98.20		6.44	110.6	
Sandstone Sand	1783	141.89	120.04	6.50	111.7	110.4
45/370	884	70.35	1	6.34	109.0	
1016 0 1	1095	87.14		6.33	108.7	 -
18 Masonry Sand	1323	105.28	96.21	6.31	108.5	109.9
45/370	1825	145.23		6.55	112.5	
	1815	144.43		6.85	117.7	_
19 Limestone	1483	118.01	126.69	6.86	117.9	117.3
Screenings 45/370	1478	117.62		6.77	116.4	

Table A11. (Continued)

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
22 TDOT 100/250	2838	225.84		6.73	115.8	
	1894	150.72	195.55	6.75	116.0	116.0
	2640	210.08		6.77	116.3	
	429	34.14	30.11	5.01	86.2	84.9
23 MBT MB AE 90	340	27.06		4.92	84.6	
	366	29.13		4.88	83.9	
0.4 W.D. G	881	70.11		5.51	94.6	
24 W.R. Grace	762	60.64	60.96	5.49	94.3	94.9
Darafill	655	52.12		5.57	95.7	
25 MBT Rheofill	547	43.53		5.23	89.8	
Limestone	531	42.26	40.80	5.23	89.9	89.4
Manufactured Sand	460	36.61		5.14	88.3	

Table A12. 637-Day EFF Compressive Strength Data

	Cor	npressive Str	ength		Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average		
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)		
	98	7.80		6.42	110.4			
1 KTC 30/300			7.80			110.4		
	213	16.95		6.78	116.5			
Mixture 2 30/370	193	15.36	15.20	6.71	115.4	115.8		
	167	13.29		6.73	115.6			
	174	13.85		6.57	112.9	_		
Mixture 3 30/440	287	22.84	16.95	6.62	113.7	112.6		
	178	14.16		6.47	111.1			
	418	33.26		6.73	115.7			
Mixture 4 45/300	699	55.62	47.80	6.80	116.8	116.4		
	685	54.51		6.79	116.8			
···	316	25.15		6.71	115.4	115.1		
5 TRMCA 45/370	483	38.44	32.63	6.73	115.6			
	431	34.30]	6.64	114.2			
	793	63.10		6.84	117.5			
Mixture 6 45/440	648	51.57	67.77	6.83	117.4	117.5		
	1114	88.65		6.84	117.6			
	522	41.54		6.33	108.8			
Mixture 7 60/300	543	43.21	47.61	6.52	112.0	109.6		
	730	58.09	-	6.29	108.0			
	1005	79.98		6.75	116.0			
Mixture 8 60/370	803	63.90	66.00	6.72	115.6	115.6		
	680	54.11		6.71	115.3			
	1387	110.37]	6.95	119.4			
9 TTU CAP 60/440	1355	107.83	107.54	6.99	120.2	119.6		
	1312	104.41	1	6.95	119.4			
10 45/050	703 55.94	55.94		6.45	110.9			
Mixture 10 45/370	585	46.55	55.12	6.45	110.8	110.7		
High Carbon Ash	790	62.87		6.42	110.4			

⁻⁻ Sample did not survive de-molding

Table A12. (Continued)

	Con	pressive Str	ength	Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
• / • • • • • • • • • • • • • • • • • •	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
	1620	128.92		6.59	113.3		
Mixture 11 60/370	1264	100.59	100.96	6.63	113.9	113.7	
High Carbon Ash	922	73.37		6.63	114.0		
	351	27.93		6.66	114.4		
Mixture 12 45/440	333	26.50	24.22	6.60	113.4	113.5	
High Carbon Ash	229	18.22		6.55	112.6		
	822	65.41		6.60	113.4		
Mixture 13 60/440	603	47.99	53.50	6.54	112.3	113.4	
High Carbon Ash	592	47.11		6.66	114.5		
Mixture 14 45/510 High Carbon Ash	809	64.38	58.04	6.78	116.6]]	
	714	56.82		6.81	117.0	116.7	
	665	52.92		6.78	116.6		
	1187	94.46	98.36	6.50	111.7	112.3	
Mixture 15 60/510	1383	110.06		6.48	111.4		
High Carbon Ash	1138	90.56		6.61	113.6		
16 Manufactured	947	75.36		6.51	111.9		
Limestone Sand	981	78.07	76.16	6.56	112.8	113.0	
45/370	943	75.04]	6.64	114.2		
17 Crushed	1166	92.79		6.39	109.9		
Sandstone Sand	1202	95.65	94.22	6.36	109.3	109.6	
45/370	1118	88.97	- 	6.37	109.6		
	937	74.56		6.21	106.7		
18 Masonry Sand	1150	91.51	83.04	6.37	109.6	108.2	
45/370	976	77.67		6.30	108.3	ļ. <u> </u>	
	1337	106.40		6.87	118.1	117.2	
19 Limestone	1549	123.27	119.07	6.83	117.4		
Screenings 45/370	1603	127.56		6.76	116.1		

Table A12. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load (lbs)	Individual (psi)	Average (psi)	Weight (lbs)	Individual (pcf)	Average (pcf)
22 TDOT 100/250	1844	146.74		6.63	113.9	
	1853	147.46	149.79	6.65	114.3	114.6
	1950	155.18		6.73	115.6	
	371	29.52	29.05	4.99	85.8	85.0
23 MBT MB AE 90	358	28.49		4.92	84.6	
	366	29.13		4.93	84.7	
	778	61.91		5.50	94.5	94.0
24 W.R. Grace	670	53.32	54.86	5.37	92.4	
Darafill	620	49.34		5.53	95.1	
25 MBT Rheofill						
Limestone]			-
Manufactured Sand			<u></u>			<u> </u>

⁻ Sample did not survive de-molding

Table A13. 637-Day EFF Lime Water Immersed Compressive Strength Data

	Con	apressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
1 KTC 30/300						
		4=				
	111	8.83		7.13	122.6	
Mixture 2 30/370	152	12.10	10.46	7.26	124.7	123.7
	*-					
	155	12.33		7.15	122.9	
Mixture 3 30/440	167	13.29	12.81	7.11	122.3	122.6
AVAILABILITY D DOV 110			1			
	516	41.06		7.66	131.7	131.9
Mixture 4 45/300	426	33.90	42.26	7.64	131.4	
	651	51.80		7.71	132.5	
	359	28.57		7.52	129.2	128.8
5 TRMCA 45/370	265	21.09	24.01	7.46	128.2	
_	281	22.36		7.50	128.9	
	609	48.46		7.62	130.9	
Mixture 6 45/440	562	44.72	49.28	7.64	131.3	131.2
	687	54.67		7.66	131.6	
	588	46.79.		7.39	127.1	<u>.</u>
Mixture 7 60/300	531	42.26	45.28	7.50	129.0	127.8
	588	46.79		7.40	127.3	
	733	58.33		7.63	131.1	
Mixture 8 60/370	682	54.27	57.32	7.70	132.3	131.7
	746	59.36		7.67	131.8	
	1607	127.88	_	7.68	132.1	_
9 TTU CAP 60/440	1602	127.48	127.68	7.67	131.9	132.0
	0	0.00]	0.00	0.0	
3.6° 4 . 10.45/270	1573	125.18		7.54	129.6	
Mixture 10 45/370	546	43.45	84.31	7.53	129.5	129.6
High Carbon Ash	0	0.00		0.00	0.0	

⁻ Sample did not survive Lime Water Immersion

Table A13. (Continued)

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)_	(lbs)	(pcf)	(pcf)
	819	65.17		7.43	127.6	
Mixture 11 60/370	1268	100.90	85.23	7.66	131.6	129.6
High Carbon Ash	1126	89.60		7.54	129.6	
	342	27.22		7.57	130.0	
Mixture 12 45/440	319	25.39	26.30	7.36	126.5	128.3
High Carbon Ash						
	762	60.64		7.56	129.9	
Mixture 13 60/440	846	67.32	63.48	7.55	129.7	129.9
High Carbon Ash	785	62.47		7.56	130.0	
	760	60.48		7.68	132.0	
Mixture 14 45/510	543	43.21	53.00	7.60	130.7	130.7
High Carbon Ash	695	55.31		7.52	129.3	
	1519	120.88		7.43	127.7	
Mixture 15 60/510	1619	128.84	131.73	7.41	127.4	127.4
High Carbon Ash	1828	145.47		7.39	127.0	
16 Manufactured	741	58.97		7.44	127.9	
Limestone Sand	801	63.74	65.65	7.66	131.6	129.8
45/370	933	74.25	1	7.56	129.9	
17 Crushed	1129	89.84		7.23	124.2	
Sandstone Sand	976	77.67	83.76	7.34	126.1	125.3
45/370	957	76.16		7.30	125.5	
	1468	116.82		7.36	126.5	
18 Masonry Sand	1139	90.64	103.73	7.28	125.1	125.9
45/370	1526	121.44		7.34	126.2	
	1520	120.96		7.78	133.8	133.6
19 Limestone	1481	117.85	114.96	7.72	132.6	
Screenings 45/370	1333	106.08		7.82	134.3	

⁻ Sample did not survive Lime Water Immersion

Table A13. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
-	1716	136.55		7.57	130.0	
22 TDOT 100/250	2446	194.65	158.68	7.45	128.1	129.5
	1820	144.83		7.59	130.4	
	253	20.13	26.23	6.03	103.7	103.2
23 MBT MB AE 90	407	32.39		5.94	102.0	
	329	26.18		6.04	103.8	
04.111.0	618	49.18		6.52	112.0	
24 W.R. Grace	654	52.04	48.28	6.47	111.2	111.7
Darafill	548	43.61		6.51	111.9	
25 MBT Rheofill	230	18.30		6.29	108.2	
Limestone	282	22.44	19.44	6.43	110.6	109.1
Manufactured Sand	221	17.59		6.31	108.5	

Table A14. 728-Day EFF Compressive Strength Data

	Cor	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
1 KTC 30/300	- -					
	-					
	111	8.83		7.13	122.6	
Mixture 2 30/370	152	12.10	10.46	7.26	124.7	123.7
					*	
	155	12.33		7.15	122.9	
Mixture 3 30/440	167	13.29	12.81	7.11	122.3	122.6
	516	41.06		7.66	131.7	131.9
Mixture 4 45/300	426	33.90	42.26	7.64	131.4	
	651	51.80		7.71	132.5	
	359	28.57	24.01	7.52	129.2	128.8
5 TRMCA 45/370	265	21.09		7.46	128.2	
•	281	22.36		7.50	128.9	
	636	50.61		6.81	117.1	
Mixture 6 45/440	604	48.06	55.7	6.68	114.8	116.6
	859	68.36		6.86	117.9	
	588	.46.79		6.42	110.4	
Mixture 7 60/300	790	62.87	52.8	6.46	111.0	111.27
	614	48.86		6.54	112.4	
	1090	86.74		6.68	114.8	
Mixture 8 60/370	945	75.20	79.3	6.68	114.8	114.71
	953	75.84		6.66	114.5	
	1561	124.22		6.76	116.2	
9 TTU CAP 60/440	1351	107.51	107.7	6.84	117.6	117.00
	1149	91.43		6.82	117.2	
N.C 10 45/270	1131	90.00	71.2	6.52	112.1	111.15
Mixture 10 45/370	624	73.53		6.34	109.0	
High Carbon Ash	631	50.21		6.54	112.4	

⁻ Sample did not survive de-molding

Table A14. (Continued)

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
11 60/070	2075	165.12		6.56	112.8	
Mixture 11 60/370	1024	81.49	110.5	6.60	113.4	113.10
High Carbon Ash	1067	84.91		6.58	113.1	
	209	16.63		6.52	112.1	
Mixture 12 45/440	190	15.12	15.9	6.58	113.1	112.60
High Carbon Ash						
10 (0/440	792	63.03		6.60	113.4	
Mixture 13 60/440	839	66.77	66.3	6.52	112.1	112.87
High Carbon Ash	868	69.07		6.58	113.1	
11.45/510	758	60.32	58.7	6.68	114.8	115.05
Mixture 14 45/510 High Carbon Ash	801	63.74		6.66	114.5	
	653	51.96		6.74	115.9	
15 (0)510	1868	132.74	126.6	6.52	112.1	
Mixture 15 60/510	1754	139.58		6.46	111.0	112.07
High Carbon Ash	1350	107.43		6.58	113.1	
16 Manufactured	836	66.53		6.46	111.0	
Limestone Sand	721	57.38	73.7	6.46	111.0	112.07
45/370	1220	97.08		6.64	114.1	
17 Crushed	1114	88.65		6.34	109.0	
Sandstone Sand	1022	81.33	85.0	6.30	108.3	108.63
45/370	1152	91.67		6.32	108.6	
10 Massam Cond	1944	154.70		6.44	110.7	
18 Masonry Sand	1263	100.51	127.6	6.28	107.9	109.67
45/370	1745	138.86		6.42	110.4	
101:	1477	117.54]	6.68	114.8	1
19 Limestone	1162	92.47	111.0	6.68	114.8	114.82
Screenings 45/370	1544	122.87		6.68	114.8	

⁻ Sample did not survive de-molding

Table A14. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	2576	204.99		6.78	116.5	
22 TDOT 100/250	2936	233.64	198.0	6.84	117.6	116.31
	1953	155.41		6.68	114.8	
	338	26.90	25.6	4.94	84.9	84.45
23 MBT MB AE 90	357	28.41		4.90	84.2	
	271	21.57		4.90	84.2	
OANUD C	688	54.75		5.32	91.4	
24 W.R. Grace	518	41.22	53.2	5.50	94.5	93.28
Darafill	799	63.58		5.46	93.9	
25 MBT Rheofill	380	30.24	, 	5.14	88.4	
Limestone	514	40.90	39.4	5.12	88.0	87.78
Manufactured Sand	591	47.03		5.06	87.0	

Table A15. 728-Day EFF Lime Water Immersed Compressive Strength Data

	Con	npressive Str	ength		Unit Weigh	t
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
		*				
1 KTC 30/300						
Mixture 2 30/370						
		==			*-	
	201	16.00		7.18	123.4	
Mixture 3 30/440	218	17.35	16.7	7.26_	124.8	124.1
	421	33.50		7.64	131.3	
Mixture 4 45/300			33.5			131.3
	359	28.57	22.2	7.56	129.9	
5 TRMCA 45/370	200	15.92		7.14	122.7	126.3
	655	52.12		7.66	131.7	130.75
Mixture 6 45/440	531	42.26	47.1	7.52	129.3	
	589	46.87		7.64	131.3	
	668	53.16		7.44	127.9	
Mixture 7 60/300	486	38.67	48.3	7.36	126.5	127.31
	668	53.16	<u> </u>	7.42	127.5	
	853	67.88		7.60	130.6	
Mixture 8 60/370	893	71.06	68.8	7.64	131.3	130.75
	846	67.32		7.58	130.3	
-	1600	127.32	_	7.70	132.4	1
9 TTU CAP 60/440	1491	118.65	116.3	7.70	132.4	132.13
	1293	102.89		7.66	131.7	
Minture 10 45/270	693	55.15		7.46	126.2	126.2
Mixture 10 45/370			55.15			
High Carbon Ash						

⁻⁻ Sample did not survive Lime Water Immersion

Table A15. (Continued)

	Cor	npressive Str	ength	Unit Weight		
Mixture	Load	Individual	Average	Weight	Individual	Average
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1135	90.32		7.46	128.2	
Mixture 11 60/370	1333	106.08	100.4	7.54	129.6	128.80
High Carbon Ash	1317	104.80		7.48	128.6	
10 15/110	360	28.65		7.50	128.9	
Mixture 12 45/440	356	28.33	29.2	7.50	128.9	128.92
High Carbon Ash	384	30.56		7.50	128.9	
3.53	914	72.73		7.50	128.9	
Mixture 13 60/440	798	63.50	68.1	7.52	129.3	129.10
High Carbon Ash						
14 45/510	643	51.17		7.54	129.6	
Mixture 14 45/510			51.2			129.6
High Carbon Ash						
16 (0/510	1336	106.32	100.9	7.60	130.6	
Mixture 15 60/510	1010	80.37		7.34	126.2	128.00
High Carbon Ash	1459	116.10		7.40	127.2	
16 Manufactured	951	75.68		7.58	129.9	129.9
Limestone Sand			75.7			
45/370					-	
17 Crushed	1268	100.90		7.30	125.5	
Sandstone Sand	1089	86.66	93.8	7.26	124.8	125.25
45/370	959	76.31		7.30	125.5	
10 Maganay Sand	1599	127.24		7.26	124.8	
18 Masonry Sand 45/370	1317_	104.80	116.0	7.26	124.8	124.80
43/3/0				***		
10 Lim	1449	115.31		7.74	133.0	
19 Limestone	1392	110.77	107.1	7.74	133.0	132.81
Screenings 45/370	1195	95.10		7.70	132.4	

⁻ Sample did not survive Lime Water Immersion

Table A15. (Continued)

	Compressive Strength			Unit Weight			
Mixture	Load	Individual	Average	Weight	Individual	Average	
	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)	
	2006	159.63		7.48	128.6	•	
22 TDOT 100/250	1957	155.73	150.3	7.44	127.9	128.11	
	1705	135.68		7.44	127.9		
	163	12.97		6.12	105.2		
23 MBT MB AE 90	115	9.15	15.1	6.04	103.8	104.28	
	291	23.16		6.04	103.8		
	618	49.18		6.56	112.8		
24 W.R. Grace	508	40.43	45.8	6.42	110.4	111.27	
Darafill	602	47.91		6.44	110.7		
25 MBT Rheofill	187	14.88		6.42	110.4		
Limestone	175	13.93	15.2	6.36	109.3	109.89	
Manufactured Sand	212	16.87	1	6.40	110.0		

APPENDIX B

Table B1. Algood, TN ZOOM! CLSM Demo – Limestone Screenings Cylinders 09/23/02

	Compressive Strength			th Unit Weight		
Age (days)	Load (lbs)	Individual (psi)	Average (psi)	Weight (lbs)	Individual (pcf)	Average (pcf)
	2159	171.76		6.26	107.60	
1	2068	164.52	168.42	6.18	106.23	106.80
İ	2124	168.97		6.20	106.57	
	4555	362.37		6.22	106.91	
7	4358	346.70	355.87	6.18	106.23	106.91
	4507	358.55		6.26	107.60	
	6218	494.67		6.12	105.19	
28	5988	476.37	512.57	6.18	106.23	105.77
	7123	566.67		6.16	105.88	
	7063	561.89		6.06	104.16	
56	7648	608.43	616.65	6.14	105.54	104.85
	8543	679.63		6.10	104.85	

Table B2. Algood, TN ZOOM! CLSM Demo - Ohio River Sand Cylinders 09/23/02

_	Compressive Strength			Unit Weight		
Age (days)	Load (Ibs)	Individual (psi)	Average (psi)	Weight (lbs)	Individual (pcf)	Average (pcf)
	1325	105.41	<u></u>	6.24	107.26	
1	1234	98.17	96.31	6.30	108.29	107.83
	1073	85.36		6.28	107.94	
	2804	223.07		6.22	106.91	
7	2885	229.51	225.06	6.30	108.29	107.60
	2798	222.59		6.26	107.60	
	4490	357.20		6.16	105.88	
28	4292	341.45	328.88	6.20	106.57	106.34
	3620	287.99		6.20	106.57	
	5532	440.10		6.18	106.23	
56	4612	366.91	431.56	6.04	103.82	105.42
	6130	487.67		6.18	6.18	<u> </u>

Table B3. Knoxville, TN ZOOM! CLSM Demo – Manufactured Limestone Sand Cylinders, 11/01/02 (Trench #1)

A	Compressive Strength			Unit Weight		
Age	Load	Individual	Average	Weight	Individual	Average
(days)	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1762	140.18		7.38	126.85	
1	1791	142.48	142.75	7.16	123.07	127.31
	1830	145.58		7.68	127.31	
	3829	304.61		7.50	128.91	
7	4081	324.66	312.81	7.44	127.88	128.34
	3886	309.15		7.46	128.26	
	5602	445.66		7.42	127.54	
28	2849	465.31	448.13	7.50	128.91	127.65
	5448	433.41		7.36	126.51	
	5832	463.96		7.22	124.10	
56	6567	522.43	491.38	7.22	124.10	123.53
	6131	487.75		7.12	122.38	

Table B4. Knoxville, TN ZOOM! CLSM Demo – Manufactured Limestone Sand Cylinders, 11/01/02 (Trench #2)

A	Compressive Strength			Unit Weight		
Age (days)	Load	Individual	Average	Weight	Individual	Average
(uays)	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1449	115.27	-	6.14	105.54	
1	1023	81.38	96.50	5.92	101.76	103.13
1	1167	92.84		5.94	102.10	
	3153	250.84		5.96	102.44	
7	2925	232.70	249.43	5.94	102.10	102.22
	3328	264.76		5.94	102.10	
	4838	384.88		N/A	N/A	
28	4731	376.37	370.91	N/A	N/A	N/A
	4418	351.47		N/A	N/A	
	5511	438.42		5.78	99.35	
56	5434	432.30	426.33	6.02	103.48	101.76
	5132	408.24		5.96	102.44	

Table B5. Nashville, TN ZOOM! CLSM Demo - Ohio River Sand Cylinders, 11/08/02 (Trench #1)

	Compressive Strength			Unit Weight		
Age (days)	Load (lbs)	Individual (psi)	Average (psi)	Weight (lbs)	Individual (pcf)	Average (pcf)
<u></u>	1317	104.77		5.92	101.76	
1	1260	100.24	101.59	5.82	100.04	100.95
	1254	99.76		5.88	101.07	
	3132	249.16		5.82	100.04	
7	2695	214.40	231.87	5.84	100.38	100.38
	2917	232.60		5.86	100.73	
	4477	356.17		5.76	99.01	
28	3494	277.96	323.97	5.72	98.32	99.01
	4246	337.79		5.80	99.69	
	5076	403.82		5.72	98.35	
56	4598	365.79	381.09	5.83	100.12	98.67
	4697	373.67		5.68	97.55	

Table B6. Nashville, TN ZOOM CLSM Demo - Ohio River Sand Cylinders, 11/08/02 (Trench #2)

_	Compressive Strength			ngth Unit Weight		
Age (days)	Load	Individual	Average	Weight	Individual	Average
(uays)	(lbs)	(psi)	(psi)	(lbs)	(pcf)	(pcf)
	1278	101.67		5.84	100.38	
1	1306	103.90	103.77	5.84	100.38	100.27
	1329	105.73		5.82	100.04	
	3099	246.54		5.84	100.38	
7	2957	235.24	245.21	5.86	100.73	100.61
	3191	253.86		5.86	100.73	
	3875	308.27		5.72	98.32	
28	3844	305.81	307.16	5.74	98.66	98.89
	3864	307.40		5.80	99.69	<u> </u>
	5674	451.39		5.73	98.56	
56	4261	338.98	406.36	5.80	99.68	98.40
	5389	428.72		5.64	96.96	

APPENDIX C

DRAFT SPECIFICATION FOR CONTROLLED LOW-STRENGTH MATERIAL

1. Scope

- 1.1 This specification covers ready-mixed controlled low-strength material (CLSM) manufactured and delivered to a purchaser in a freshly mixed and unhardened state as hereinafter specified. Requirements for quality of CLSM shall be either or as hereinafter specified. This specification does not cover placement, curing, or protection of CLSM after delivery to the purchaser.
- 1.2 The values stated in inch-pound units are regarded as standard.
- 1.3 As used throughout the specification the manufacturer shall be the contractor, subcontractor, supplier, or producer who furnishes the ready-mixed CLSM. The purchaser shall be the owner or representative thereof.
- 1.4 The text of this standard references notes and footnotes which provide explanatory material. These notes and footnotes (excluding those in tables) shall not be considered as requirements of the standard.

2. Referenced Documents

ASTM D 4832 Test Method for Preparation and Testing of Soil-Cement Slurry Test Cylinders

ASTM D 5971 Standard Practice for Sampling Freshly Mixed Controlled Low-Strength Material

ASTM D 6023 Standard Test Method for Unit Weight, Yield, Cement Content, and Air Content (Gravimetric) of Controlled Low Strength Material (CLSM)

ASTM D 6024 Standard Test Method for Ball Drop on Controlled Low Strength Material (CLSM) To Determine Suitability for Load Application

ASTM D 6103 Standard Test Method for Flow Consistency of Controlled Low Strength Material (CLSM)

Tennessee Department of Transportation (TDOT), Standard Specifications for Road and Bridge Construction

3. Basis of Purchase

- 3.1 The basis of purchase shall be the cubic yard of freshly mixed and unhardened CLSM as discharged from the mixer.
- 3.2 The volume of freshly mixed and unhardened CLSM in a given batch shall be determined from the total mass of the batch divided by the mass per unit volume of the CLSM. The total mass of the batch shall be calculated either as the sum of the masses of all materials, including water, entering the batch or as the net mass of CLSM in the batch as delivered. The mass per unit volume shall be determined in accordance with Test Method D 6023 from the average of at least three measurements. Each sample shall be taken in accordance with D 5971.

Note 1 - It should be understood that the volume of hardened CLSM may be, or appear to be, less than the expected due to waste, spillage, over-excavation, movement of forms, and some loss of entrained air or subsidence, none of which are the responsibility of the producer.

4. Ordering Information

- 4.1 The purchaser shall specify which of options A, B, or C shall be used as a basis for determining the proportions of the CLSM to produce the required quality. As defined herein, there are four (4) types of CLSM: excavatable flowable fill, airentrained excavatable flowable fill, general use CLSM, and early strength flowable fill.
 - (A) Excavatable flowable fill (EFF) When specified on the plans, an EFF shall be designed, proportioned, and delivered to the project meeting the following performance requirements:

Property	Specification Limit
Load Application (ASTM D6024)	24 hours maximum in any condition
Consistency (ASTM D 6103)	8" minimum
Compressive strength (ASTM D 4832) ¹	30-psi minimum at 28-days
Compressive strength (ASTM D 4832) 1	110-psi maximum at 98-days or 160-psi maximum at 364-days

1- ASTM D 4832 procedure as modified in section 6

Specifiers should be aware that EFF designs will take longer periods of time to have sufficient bearing strength before backfilling is allowed. Each consistency test shall represent up to one hundred cubic yards of flowable fill at each installation.

The ready mix producer must complete a test trench (at least 3' wide x 3'deep x 8' long), during the mixture design process, with the proportioned materials to demonstrate the mixture will meet the performance criteria. The mixture

design may be adjusted with approval by the Engineer to obtain the required properties.

(B) Air-entrained Excavatable flowable fill (AEFF) – When specified on the plans, an EFF shall be designed, proportioned, and delivered to the project meeting the following performance requirements:

Property	Specification Limit
Air content (ASTM D 6023)	Minimum 20 percent
	Maximum 30 percent
Load Application (ASTM D6024)	24 hours maximum in any condition
Consistency (ASTM D 6103)	8" minimum
Compressive strength (ASTM D 4832) ¹	30-psi minimum at 28-days
Compressive strength (ASTM D 4832) ¹	110-psi maximum at 98-days or
Compressive suchgur (ASTM D 4832)	185-psi maximum at 364-days

¹ - ASTM D 4832 procedure as modified in section 6

Specifiers should be aware that EFF designs will take longer periods of time to have sufficient bearing strength before backfilling is allowed. Each consistency test shall represent up to one hundred cubic yards of flowable fill at each installation.

The ready mix producer must complete a test trench (at least 3' wide x 3'deep x8' long), during the mixture design process, with the proportioned materials to demonstrate the mixture will meet the performance criteria. The mixture design may be adjusted with approval by the Engineer to obtain the required properties.

Note 2 — Air-entrained EFF mixtures have little or no bleeding and do not appreciably consolidate and subside. Air-entrained EFF mixtures may be preferable where bleed water could cause problems or conditions impede bleed water loss.

(C) General Use CLSM: When not otherwise specified in the plans, or Contract, all flowable fill_shall be general use and shall be proportioned to meet the following:

Property	Specification Limit
Load Application (ASTM D6024)	24 hours maximum in any condition
Consistency (ASTM D 6103)	8" minimum

Note 3 - The above proportions may be adjusted by the Engineer to obtain the consistency required for satisfactory flow. Each consistency test shall represent up to one hundred cubic yards of flowable fill at each installation.

(D) **ZOOM!** Early strength flowable fill (ESFF) - When specified on the plans ESFF shall be designed, proportioned, and provided that meets the following performance specifications:

Property	Specification Limit	
Air content (ASTM D 6023)	Maximum 30 percent ¹	
Load Application (ASTM D6024)	6 hours maximum in any condition	
Consistency (ASTM D 6103)	8" minimum	
Compressive strength (ASTM D 4832) ²	30-psi minimum in 24-hours	

1- when using air entrained mixture design

The proportions of ESFF shall include sufficient amounts of cementitious materials, high range water reducers, accelerators, and other chemical admixtures so the ESFF can be loaded/backfilled as specified. Each consistency test shall represent up to one hundred cubic yards of flowable fill at each installation.

The ready mix producer must complete a test trench (at least 3' wide x 3'deep x 8' long), during the mixture design process, with the proportioned materials to demonstrate the mixture will meet the performance criteria. The mixture design may be adjusted with approval by the Engineer to obtain the required properties.

The Contractor shall furnish certification that all flowable fill delivered to the project has been batched in accordance with the approved mixture design and meets the designated properties as specified. Chemical Additives and/or Air Entraining Admixture may be used to produce the desired consistency with no additional payment to be made for such additives.

²⁻ ASTM D 4832 procedure as modified in section 6

5. Materials

5.1 Materials used in the placement of CLSM (flowable fill) shall meet the following requirements:

<u>Material</u>	<u>Subsection</u>
Fine Aggregate	903.01(f) except gradation shown below
Portland Cement, Type I	901.01
Fly Ash, Class C or Class F	918.31
Ground granulated blast furnace slag (GGBFS)	918.32
Water	918.01
Chemical Additives	918.09
Air Entraining Admixtures	918.09

CLSM Aggregate Gradation Requirements

Sieve Size	Total Per Cent Passing by Weight
3/8-inch	100
No. 200	0-21

6. Modifications to ASTM D 4832-95^{ε1}

- 6.1. Change the preferred mold from 6-in. diameter, 12-in. high rigid plastic (ASTM D 4832-95^{ε1} Section 6.1) to 4-in. diameter, 8-in. high single-use, wax-coated cardboard.
 - Note 4 Past research at Tennessee Technological University has shown that less cylinder damage occurs during demolding when using cardboard molds.
- 6.2. Delete the last sentence of ASTM D 4832-95⁸¹ Note 4 and the second sentence of Section 10.1.
 - Note 5 Past research at Tennessee Technological University has shown that EFF cylinders are far too easy to damage to allow mounding of the top and subsequent removal of the mound with a wire brush.
- 6.3. Insert 10.1.4. The preferred capping method for EFF is wet suit neoprene restrained in rigid retainers as described in Sauter, H. J. and Crouch, L. K., "An Improved Capping Technique for Excavatable Controlled Low Strength Material Compressive Strength Cylinders," Journal of Testing and Evaluation, JTEVA, Vol.28, No.3, May 2000, pp. 143-148.
 - Note 6 The research described in the paper indicates that the recommended capping technique yields more realistic compressive strengths than the ASTM D 4832-95 approved methods. In addition, the recommended method is either statistically or logistically superior (in most cases both) to the ASTM D 4832-95 approved capping methods.